

Town of Smiths Falls
Public Works Operation Center
11 Air Care Drive
Smiths Falls ON, K7A 0B2

Date: July 20, 2026

PART 1 - General

- .1 This Addendum is issued prior to tender closing to provide revisions to the Contract Documents.
- .2 Such revisions shall become part of the Contract Documents and shall change the original contract documents only in the manner and extent stated.
- .3 Bidders shall take this Addendum into Consideration when preparing and submitting a Bid and shall acknowledge receipt of this Addendum in their bids.

PART 2 – Contract Documents

Drawings

- .1 Refer to the attached Robinson Land Development Civil drawings (2 pages 24" x36")
 - .1 Include these drawings in the previously issued drawing package
- .2 Refer to the attached D+M structural engineering structural drawings (11 pages 24"x36")
 - .1 Include these drawings in the previously issued drawing package

Specifications

- .1 Refer to Specification **Section 00 73 00 Supplementary General Conditions**
 - .1 Add this section in to the previously issued specifications
- .2 Refer to the attached Geotechnical Report prepared by Cambium.
 - .1 Add this geotechnical report as an appendix to the previously issued Specification package.

Questions

Question 1: Please provide a copy of the Supplementary Conditions to the CCDC 2 (2020) Contract that will be used as the form of agreement.

Answer1: refer to the attached specification section 00 73 00 – Supplementary General Conditions.

Question 2: Please confirm if the Cash Allowances identified in 01 21 00 Items 1.2.7.1 to .4 should be entered under line 16 of the Section D breakdown or provide an updated breakdown including the Cash Allowances.

Answer 2: Yes please enter these amounts in line 16 of the tender breakdown.

Question 3: Please confirm if a Site Visit or Bidder's Briefing will be held for this project?

Answer 3: No site visit or bidders briefing is planned for this project

Question 4: Please provide the Geotechnical report for the site and confirm if any contaminated soils or materials are anticipated?

Answer 4: Attached to this addendum is the geotechnical report.

Question 5: Please provide Structural and Civil drawings for this project.

Answer 5: Attached to this addendum are the structural and civil drawings

Question 6: Wall & Roof Panel Systems

a) Currently on drwg. A401, it shows for Wall/Roof Legend Notes, Symbols MR1, MC1 & MC2 that only Robertson profile type panels are to be used. Please confirm that a Westman, Vicwest or Agway similar profile panel is acceptable & approved.

Answer 6: Yes alternates meeting the same criteria as the products specified will be accepted.

Question 7: Office Bldg. (G/L's 1-3, B-F) Drwg. A203

b) Advise if the attached Office Building (G/L's 1-3, B-F) can be designed from its current design with a "Flat Roof" and membrane built-up roof system to a low slope pre-engineered metal building roof design. Roof design would be a (SSR) insulated roof to accommodate the required roof openings as well as the design weights for the RTU's and interior collateral loads as required and as shown on drawing A203 and M420.

Answer 7: This revision will not be accepted.

Question 8: Please confirm that as long as specifications are met, alternate systems to Robertson may be used, such as Matrix, Bay-Shore, Canadian Metal Buildings, etc.

Answer 8: Alternate pre-engineered building manufacturers will be accepted. The alternate manufacturers proposed system must match the design as it is presented in the drawings. Any major change to design and structural arrangement as it has been shown in the drawings will not be considered as equivalent and would not be approved.

Question 9: Section 02 43 13-Selective Demolition: 1.10 Site Conditions, indicates hazardous materials are NOT expected to be encountered. However, it also refers to Section 02 26 00-Hazardous Material Assessment, which is missing from the specification. Please provide the Hazardous Material Assessment.

Answer 9: Disregard the reference to a hazardous material assessment identified in section 02 26 00.

Question 10: The door Spec is asking for G-2020 which is not comparable to the Garex A175 or Richard Wilcox T175. The correct Garaga Door should be the G-5000.

The Garex also doesn't meet the thickness requirement stated in the written spec of 45mm. Are we able to supply the Garaga G5000 to match the Spec.

Answer 10: The Garage G5000 is considered as the approved equivalent. Remove the reference of G-2020 from the specification.

Question 11: I'm reaching out to request that Automated Logic be included as an approved BAS controls vendor for this tender. Automated Logic has been a leading provider of automation and control systems in the Ottawa area for over 25 years. We are the manufacturer of our own products, enabling us to provide exceptional service and ongoing support. Utilizing BACnet open protocol, our system seamlessly integrates with a wide range of mechanical equipment and building systems.

Answer 11: Automated logic is considered as an approved alternate BAS supplier.

Question 12: Please consider extending this project if the date for last addenda is July 28th, and it closes on the

30th, we need sufficient time to ensure all subs are able to review the last addenda and properly revise quotes especially for a project of this size.

Answer 12: The current proposed dates will remain as is.

Question 13: Is there more information on the existing building to be demolished? The spec says concrete foundations, below grade construction and utilities to be disconnected, capped and sealed, but there is nothing to show quantities of these items. Please advise

Answer 13: The existing building to be demolished is constructed as a pre-engineered steel building, with traditional concrete frost wall foundations and piers around the perimeter and piers within the foot print of the building. Their depth is unknown but each bidder is to assume a depth of 1800mm as their height and a full removal to that depth.

Question 14: Ad101 says remove misc site material and debris, what is this? Since there is no job showing this should be clarified and not left for assumption

Answer 14: the material noted in those areas consists of piles of soils and granular, some scrap wood, grasses trees and shrubs.

Question 15: Please confirm a building permit has been obtained.

Answer 15: A Building permit will be obtained by the owner at their expense.

Question 16: Please confirm project commencement and any milestone dates for project.

Answer 16: Project commencement will be after the tender close and approval is given to proceed to the successful bidder. Each bidder is to provide a schedule of anticipated construction duration. No other specific milestone dates are exist.

Question 17: Please confirm if there will be a closing date extension.

Answer 17: The dates will remain as is

Question 18: Please confirm if a hydro layout has been submitted and scheduled with Hydro One.

Answer 18: Yes Hydro One has been engaged.

Question 19: RFI-Masonry-Structural Please clarify details for masonry walls such as rebar and grouting required as none is provided.

Answer 19: Details for block wall reinforcing are provided in the attached structural drawings

Question 20: 1/A204 and 3/A204 both show their slab type, 2/A204 does not show a slab type, what slab type is the Cold Storage?

Answer 20: Slab construction in the cold storage building is shown on the attached structural drawings.

Question 21: Damproofing- detail 1A601 noted damproofing to climb over foundation wall onto plywood sheathing. But typically foundation is already dampproofed and backfilled before exterior wall construction begins. Please clarify if this is the detail required.

Answer 21: yes this detail is required.

Question 22: Sprinklers-We see that there are sprinklers in the new operation building but non for the new storage building. is this correct or should we include sprinkler for that building as well. Please clarify.

Answer 22: Sprinklers are not required In the cold storage building

Question 23: Does the existing building have fire protection system to be demolished?

Answer 23: The existing building is not sprinklered.

Question 24: Will the architect accept Fire Platinum F or L from Glassopolis for the fire rated glazing.

Answer 24: Glass alternates meeting the same criteria as the specified products will be considered.

Question 25: Section D asks for the price of the Pre Engineered Main Building and the Cold storage split up assuming for budget purposes, but then you ask for the other subcontractor pricing but not per building. The only portion of this sheet that will properly show a per building cost is the Pre engineered building section, as the rest of the sub pricing for both buildings will be added together on this sheet. Is this intentional?

Answer 25: yes

End of Addendum

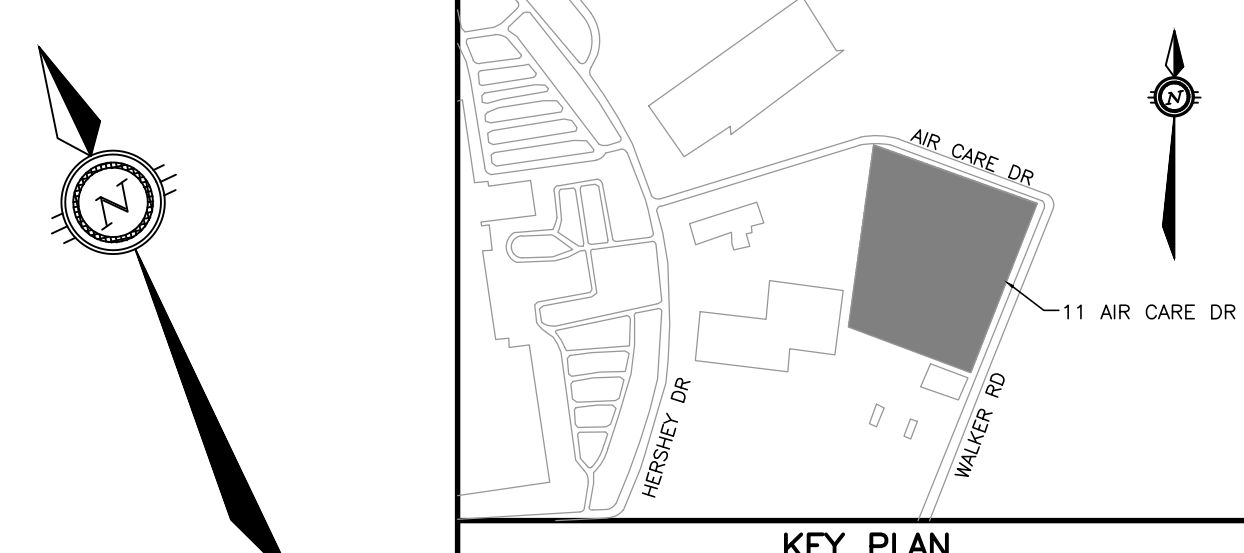
Attachments:

Drawings (24"x36"):

- Robinson Land Development – Civil Drawing package (2 pages)
- D+M structural engineering - Structural Drawing package (11 pages)

Specification Sections (8.5x11" unless otherwise noted)

- 00 73 00 – Supplementary General Conditions (1 page)
- Cambium – Geotechnical Investigation Report – 11 Air Care Drive, Smiths Falls Ontario (38 pages)

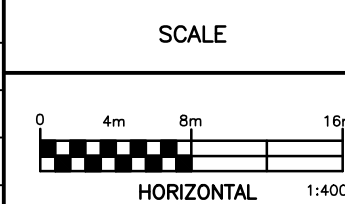


- NOT FOR CONSTRUCTION

NOTES

THE POSITION OF ALL POLE LINES, CONDUITS, WATERMAINS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

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						HORIZONTAL 1:400
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NO.	REVISION DESCRIPTION			DATE	BY	



Robinson
Land Development

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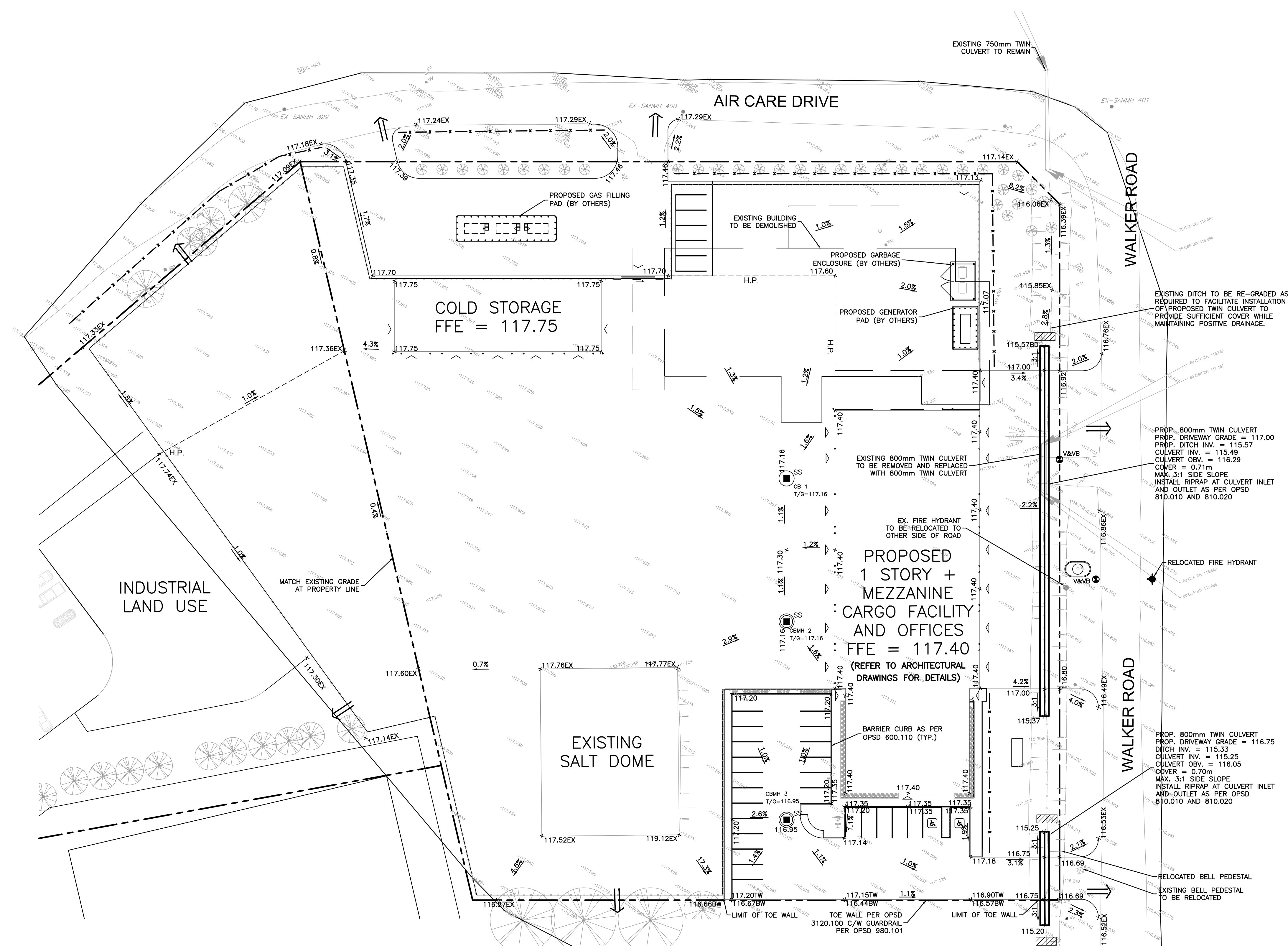
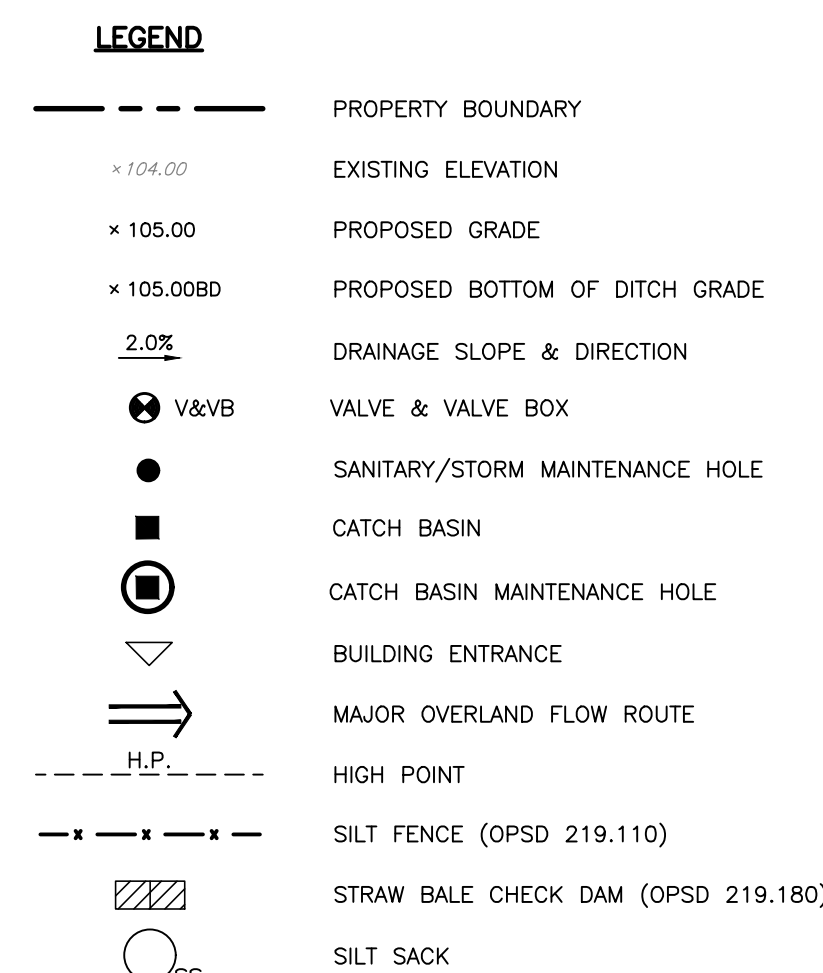
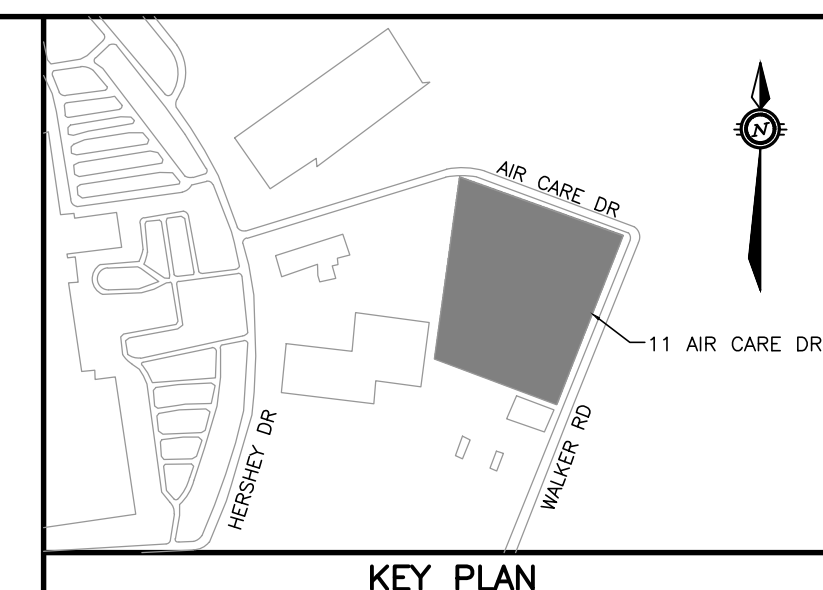
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TOWN OF SMITHS FALLS

11 AIR CARE DRIVE,
SMITHS FALLS, ON

SITE SERVICING PLAN

PROJECT No.	26061
SURVEY	TOSF
DATED	JULY 2026
DWG. No:	26061-S1

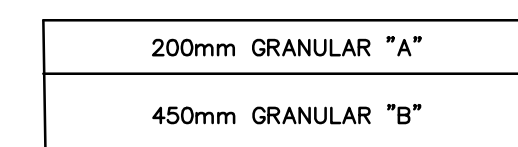
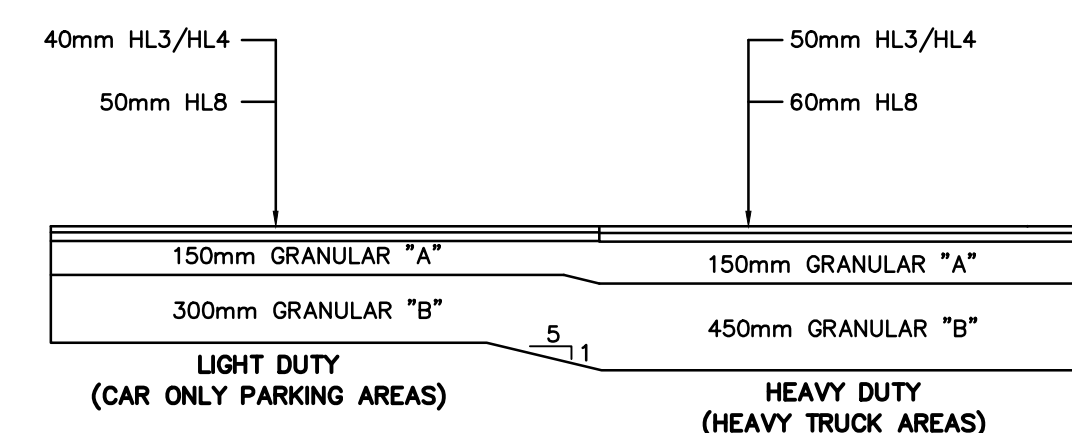


EROSION & SEDIMENT CONTROL NOTES:

1. THE CONTRACTOR SHALL IMPLEMENT BEST MANAGEMENT PRACTICES TO PROVIDE FOR PROTECTION OF THE AREA DRAINAGE SYSTEM AND THE ULTIMATE RECEIVING WATERCOURSE DURING CONSTRUCTION ACTIVITIES. THE CONTRACTOR ACKNOWLEDGES THAT FAILURE TO IMPLEMENT APPROPRIATE EROSION AND SEDIMENT CONTROL MEASURES MAY BE CAUSAL IN ANY FUTURE LITIGATION AND/OR DISAPPLICABLE REGULATION AGENCY.
2. LIMIT THE EXTENT OF EXPOSED SOILS AT ANY GIVEN TIME.
3. EROSION AND SEDIMENT CONTROL MEASURES SHALL BE MAINTAINED UNTIL VEGETATION HAS BEEN RE-ESTABLISHED OR RE-ESTABLISHMENT IS NOT FEASIBLE OR OTHERWISE DISTURBED AREAS AS SOON AS POSSIBLE.
4. STOCKPILE SOIL AWAY (15 METRES OR GREATER) FROM WATERCOURSES, DRAINAGE FEATURES AND TOP OF STEEP SLOPES.
5. SLOPE AREAS TO BE PLACED UNDERNEATH THE FRAME AND COVER OF ALL PROPOSED AND EXISTING CATCH BASIN AND OPEN COVER STORM MANHOLES UNTIL CONSTRUCTION IS COMPLETED.
6. SEDIMENT BARRIER SHALL BE INSTALLED AS PER OPSD 219.110 WHERE INDICATED AND MAINTAINED AS REQUIRED.
7. INSTALL MUD MATS AT ALL CONSTRUCTION ENTRANCES.
8. EROSION AND SEDIMENT CONTROL MEASURES SHALL BE UNDERTAKEN ON A WEEKLY BASIS AND AFTER MAJOR STORM EVENTS (>25mm rain in 24 HOURS PERIOD) ON SEDIMENT CONTROL BARRIERS AND ANY DAMAGE REPAIRED IMMEDIATELY.
9. EROSION AND SEDIMENT CONTROL MEASURES SHALL ALSO BE ASSESSED (AND REPAIRED AS REQUIRED) FOLLOWING SIGNIFICANT SNOWMELT EVENTS.
10. VISUAL INSPECTIONS SHALL ALSO BE UNDERTAKEN IN ANTICIPATION OF LARGE STORM EVENTS (OR A SERIES OF SMALLER EVENTS) THAT COULD POTENTIALLY YIELD SIGNIFICANT RUNOFF VOLUMES.
11. CARE SHALL BE TAKEN TO PREVENT DAMAGE TO EROSION AND SEDIMENT CONTROLS DURING CONSTRUCTION OPERATIONS.
12. IN SOME CASES, BARRIERS MAY BE REMOVED TEMPORARILY TO ACCOMMODATE THE CONSTRUCTION OPERATIONS, THE AFFECTED BARRIERS SHALL BE REINSTITATED IMMEDIATELY AFTER CONSTRUCTION OPERATIONS ARE COMPLETED.
13. EROSION AND SEDIMENT CONTROL MEASURES SHALL BE ADJUSTED AS REQUIRED AS THE SITE BECOMES DEVELOPED.
14. EROSION CONTROL DEVICES SHALL BE CLEANED OF ACCUMULATED SEDIMENTATION AS REQUIRED AND REPLACED AS NECESSARY.
15. DURING THE COURSE OF CONSTRUCTION, IF THE ENGINEER BELIEVES THAT ADDITIONAL PREVENTION METHODS ARE REQUIRED FOR EROSION CONTROL AND SEDIMENTATION, THE CONTRACTOR SHALL IMPLEMENT ADDITIONAL MEASURES, AS REQUIRED, TO THE SATISFACTION OF THE ENGINEER.
16. CONSTRUCTION AND MAINTENANCE REQUIREMENTS FOR EROSION AND SEDIMENT CONTROLS ARE TO COMPLY WITH THE 2018 SEDIMENT CONTROL BEST MANAGEMENT PRACTICES (BMP) MANUAL.

GRADING NOTES:

1. CONCRETE CURB SHALL BE IN ACCORDANCE WITH OPSD 600.110. PROVISION SHALL BE MADE FOR CURB DEPRESSIONS AT SIDEWALKS AND DRIVEWAYS.
2. CONCRETE SIDEWALK SHALL BE FINISHED ABOVE FINISHED ASPHALT GRADE UNLESS OTHERWISE NOTED.
3. CONCRETE SIDEWALK SHALL BE IN ACCORDANCE WITH OPSD 310.010, CANT MONILITHICALLY WITH CURB AS REQUIRED.
4. PROVIDE A PERMANENT SILT SACK (ADS FLEXSTORM OR APPROVED EQUIVALENT) IN THE CATCHBASIN AND ALL EXISTING MANHOLE COVERS.
5. ASPHALT WEAR COURSE SHALL NOT BE PLACED UNTIL THE VIDEO INSPECTION OF SEWERS & NECESSARY REPAIRS HAVE BEEN CARRIED OUT TO THE SATISFACTION OF THE ENGINEER.
6. ALL EXISTING OR DISTURBED PAVEMENT SHALL BE SAWN-CUT TO FORM A NEAT AND STRAIGHT LINE PRIOR TO PLACING NEW ASPHALT.
7. ALL FOUNDATION AND STRUCTURES RECOMMENDATION BY THE GEOTECHNICAL ENGINEERS ARE AS FOLLOWS:

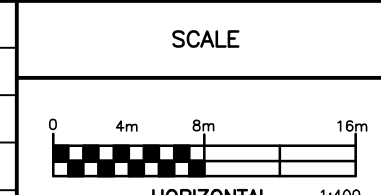


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APPROVED	SM

TOWN OF SMITHS FALLS

11 AIR CARE DRIVE,
SMITHS FALLS, ON

GRADING PLAN

PROJECT No.	26061
SURVEY	TOSF
DATED	JULY 2026
DWG. No:	26061-GB1

FOUNDATIONS

GEOTECHNICAL CONDITIONS

- ALL FOOTINGS TO BEAR ON APPROVED BEARING SURFACE WITH MINIMUM DESIGN BEARING STRENGTHS NOTED BELOW (OR IN THE FOUNDATION SCHEDULE, IF APPLICABLE).
- ALL BEARING CONDITIONS MUST BE REVIEWED ON SITE BY THE GEOTECHNICAL ENGINEER TO CONFIRM ADEQUATE BEARING CONDITIONS BEFORE PLACING CONCRETE.
- REFERENCE GEOTECHNICAL REPORT: GEOTECHNICAL INVESTIGATION REPORT - 11 AIR CARE DRIVE, SMITHFALLS ONTARIO, DATED JUNE 30 2026, CAMBIUM REFERENCE: 0265058.000.
- DESIGN BEARING STRENGTHS:

SERVICEABILITY LIMIT STATE (SLS):	N/A
ULTIMATE LIMIT STATE (ULS):	600 kPa
- PROVIDE MINIMUM FROST COVER TO UNDERSIDE OF FOUNDATIONS AS RECOMMENDED IN GEOTECHNICAL REPORT OR LISTED BELOW (WHICHEVER IS GREATER):

PERIMETER OF HEATED BUILDING:	1600mm
ISOLATED EXTERIOR FOOTINGS:	1600mm

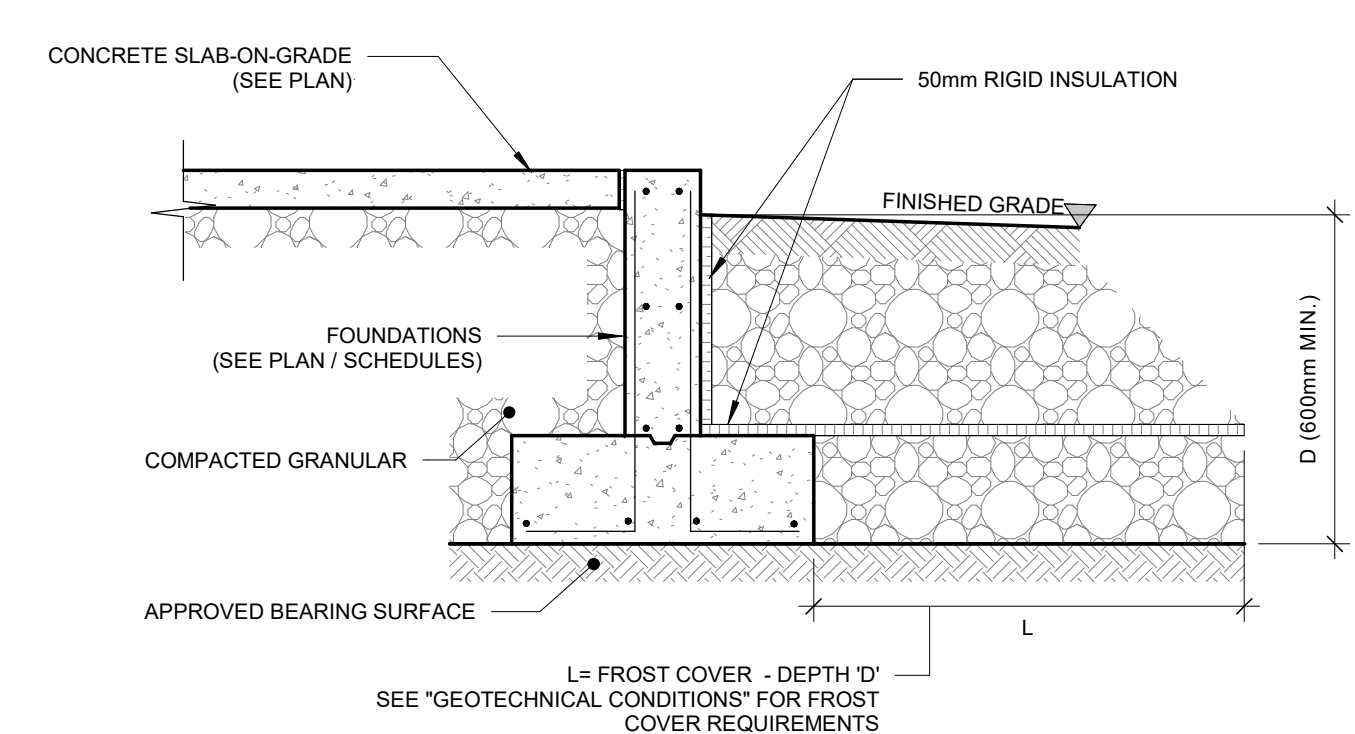
EXCAVATION

- PROTECT SUBGRADE FROM FREEZING AND FROST ACTION AT ALL TIMES DURING CONSTRUCTION. DO NOT POUR CONCRETE ON FROZEN GROUND OR GROUND THAT HAS FROZEN.
- PRIOR TO ANY EXCAVATION OR PILING OPERATION, VERIFY LOCATION OF EXISTING SERVICES AND TAKE ALL NECESSARY MEASURES TO PROTECT AND MAINTAIN SERVICES. NOTIFY OWNER AND ENGINEER IF ANY SERVICES NOT SHOWN ON CIVIL PLANS OR OTHERWISE EXPECTED ARE ENCOUNTERED. DO NOT PROCEED FURTHER UNTIL DIRECTED.
- CARE MUST BE TAKEN TO AVOID UNDERMINING EXISTING BUILDING FOUNDATIONS OR UNDERGROUND SERVICES.
- IF EXISTING BUILDING FOUNDATIONS OR UNDERGROUND SERVICES ARE PRESENT WITHIN OR ADJACENT TO THE WORK AREA, CONFIRM LOCATION AND DEPTH BEFORE PROCEEDING WITH EXCAVATION.
- UNLESS OTHERWISE OUTLINED IN GEOTECHNICAL REPORT, DO NOT EXCAVATE BELOW A LINE EXTENDING DOWNWARD FROM EXISTING FOUNDATIONS AT A SLOPE OF 1 VERTICAL TO 2 HORIZONTAL.
- SIDES AND BASE OF EXCAVATION MUST BE FREE OF WATER AND LOOSE OR REMOLDED MATERIAL BEFORE PLACING CONCRETE.
- IF DEWATERING OPERATIONS ARE REQUIRED, DESIGN AND COORDINATE DEWATERING OPERATIONS TO PREVENT DAMAGE AND/OR SETTLEMENT OF ADJACENT FOUNDATIONS.

BACKFILLING

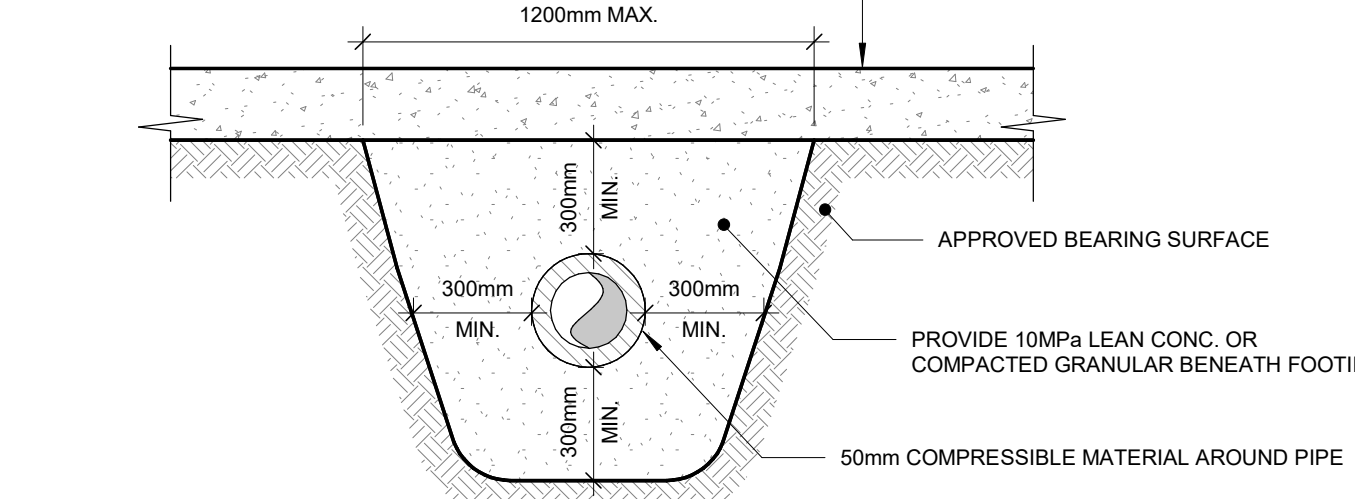
- RE-USE OF EXISTING MATERIAL IS SUBJECT TO REVIEW AND ACCEPTANCE BY THE PROJECT GEOTECHNICAL ENGINEER.
- ENSURE 75% OF CONCRETE DESIGN STRENGTH HAS BEEN ATTAINED PRIOR TO BACKFILLING AGAINST STRUCTURES.
- ENSURE ADEQUATE LATERAL SUPPORT IS PROVIDED TO FOUNDATION WALL PRIOR TO BACKFILLING.
- BACKFILL TO FOUNDATION WALLS AND WITHIN BUILDING FOOTPRINT AS RECOMMENDED IN GEOTECHNICAL REPORT. IF NOT NOTED, BACKFILL WITH OPSS GRANULAR B TYPE 2 MATERIAL IN LIFTS NOT EXCEEDING 300mm, COMPACTED TO MINIMUM 95% STANDARD PROCTOR MAXIMUM DRY DENSITY (SPMDD). FINAL LIFT BELOW SLAB-ON-GRADE TO BE 200mm OF OPSS GRANULAR A, COMPACTED TO MINIMUM 98% SPMDD.

ALTERNATE FROST PROTECTION



MECHANICAL INTERFERENCES

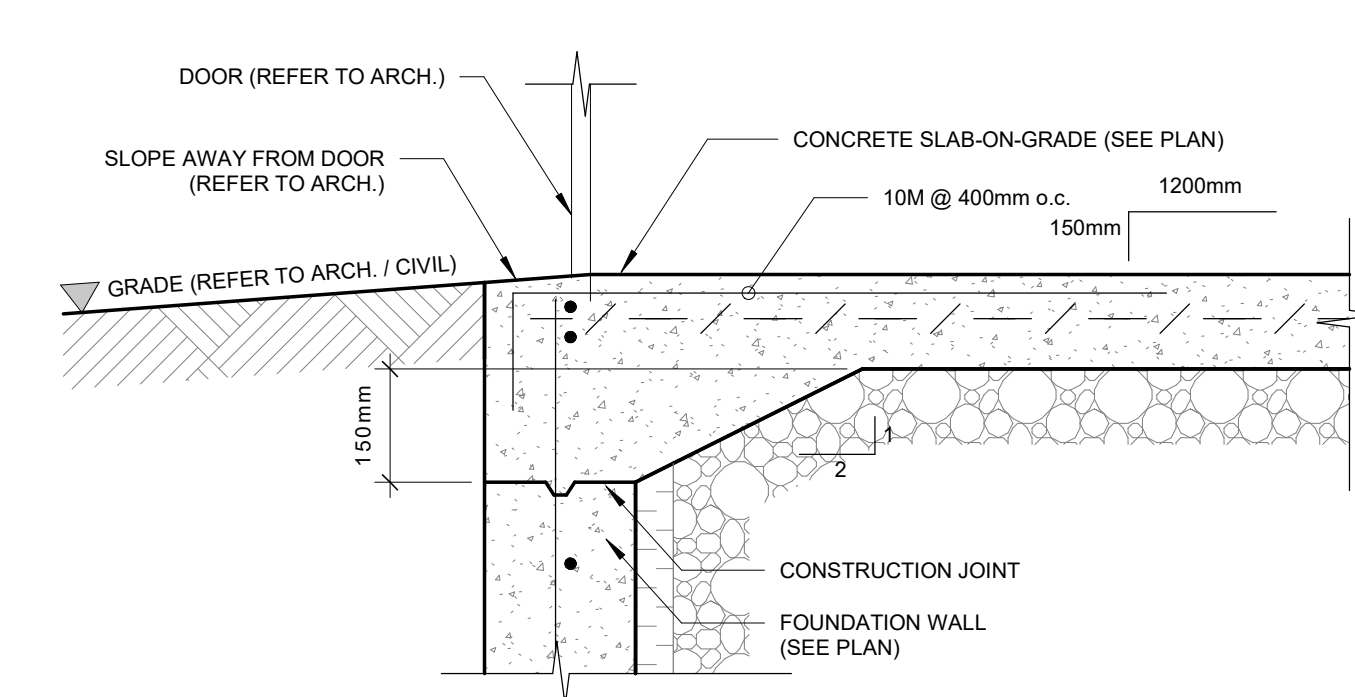
PIPE PERPENDICULAR UNDER STRIP FOOTING



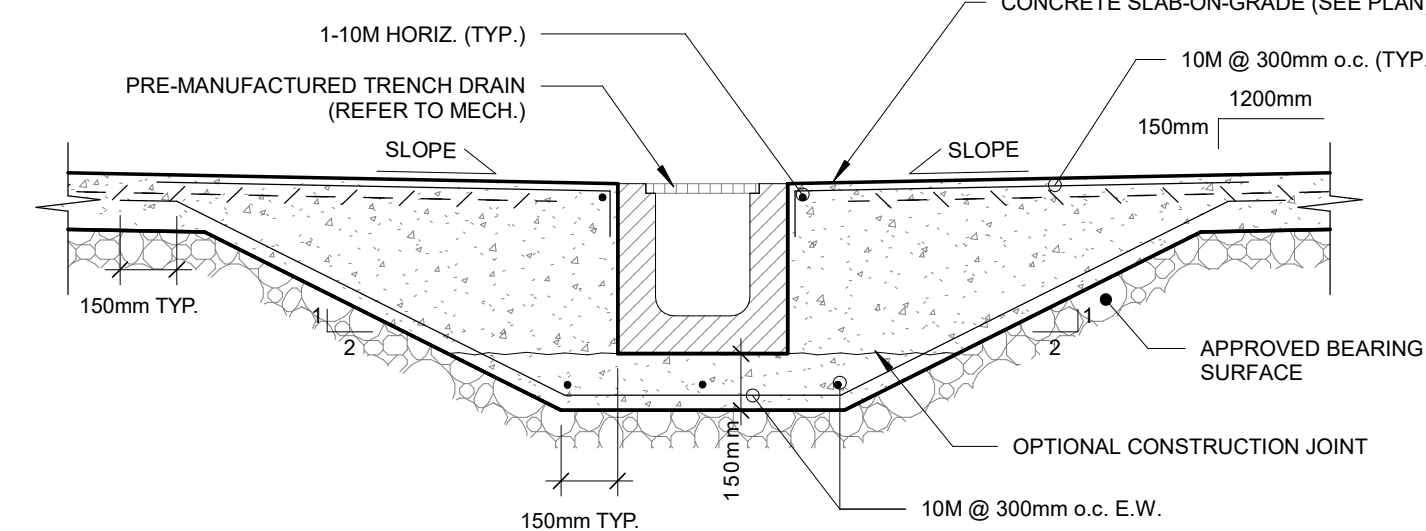
NOTES:

- PIPE NOT TO CROSS BELOW CONCENTRATED POINT LOAD ABOVE. ALL PIPES CROSSING BELOW STRIP FOOTING TO BE REVIEWED BY ENGINEER PRIOR TO PLACING.

SLAB-ON-GRADE AND FOUNDATION WALL AT DOOR

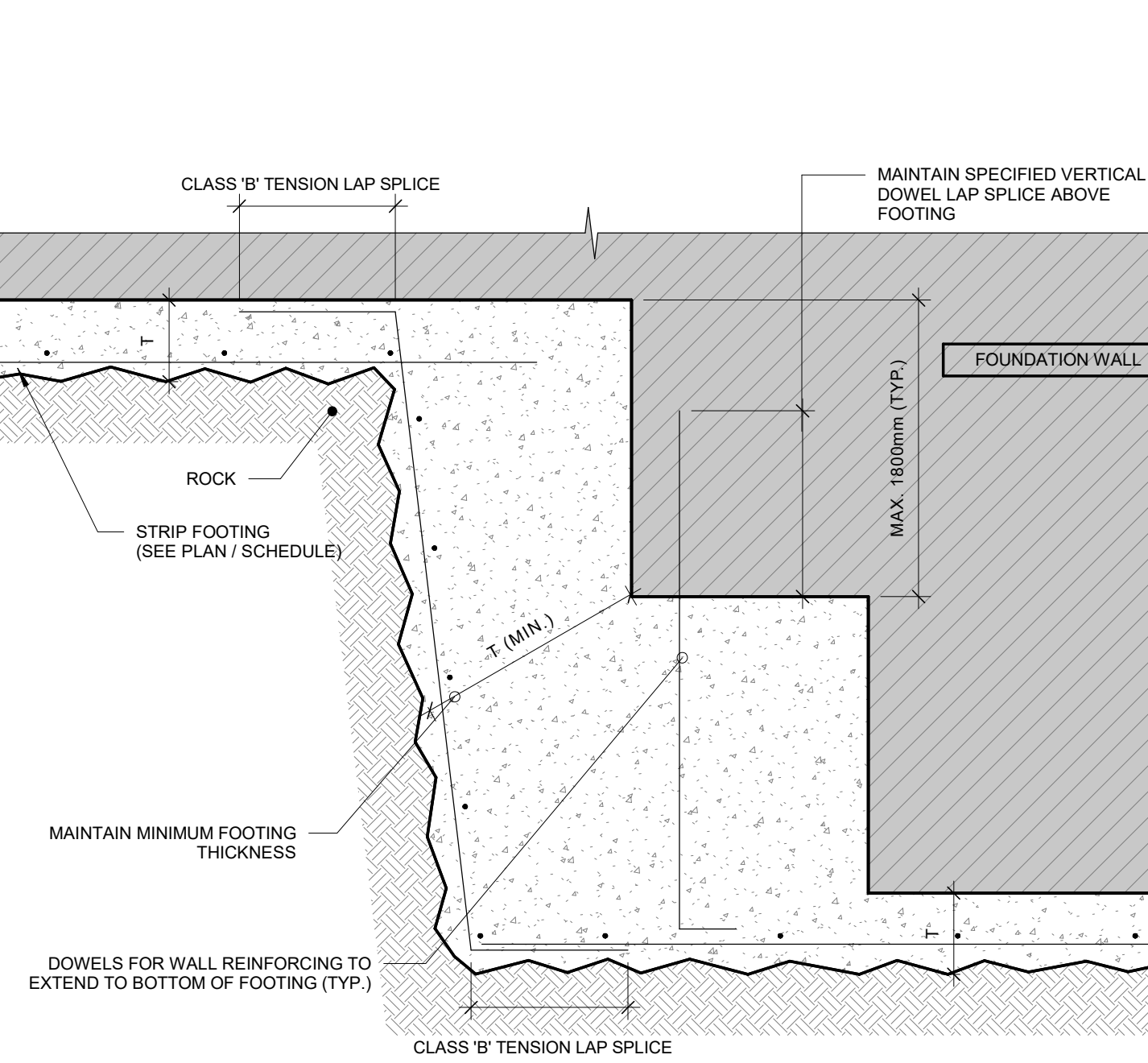


TYPICAL TRENCH DRAIN



STEPPED FOOTINGS

STEPPED FOOTING ON ROCK



CONCRETE

GENERAL

- ALL CONCRETE WORK, INCLUDING MATERIALS, MIXING, PLACING, CURING AND FORMWORK SHALL BE IN ACCORDANCE WITH CSA A23.1.
- TESTING OF CONCRETE AND CONCRETE MATERIALS SHALL BE IN ACCORDANCE WITH CSA A23.2.
- FALSEWORK AND FORMWORK SHALL BE IN ACCORDANCE WITH CSA S269.1-16 (R2021).
- UNLESS NOTED OTHERWISE, ALL EXPOSED CORNERS SHALL BE FINISHED WITH 20mm CHAMFER.
- REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SIZES AND LOCATIONS OF ALL EQUIPMENT BASES, HOUSEKEEPING PADS, PITS, SUMPS, TRENCHES, DEPRESSIONS, CURBS, CHAMFERS AND SLOPES NOT SHOWN ON STRUCTURAL DRAWINGS.
- MAINTAIN MINIMUM SLAB THICKNESS INDICATED AT SLOPES, DEPRESSIONS AND CHANGES IN ELEVATION UNLESS NOTED OTHERWISE.

REINFORCING STEEL

- REINFORCING STEEL SHALL BE DEFORMED STEEL BARS IN ACCORDANCE WITH CSA G30.18, GRADE 400R.
- WHERE SPECIFIED, EPOXY COATING SHALL BE IN ACCORDANCE WITH ASTM A775.
- WELDED STEEL WIRE MESH SHALL BE IN ACCORDANCE WITH ASTM A1064/A1064M (FLAT SHEETS ONLY).
- ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED, PLACED AND SUPPORTED IN ACCORDANCE WITH THE REINFORCING STEEL INSTITUTE OF CANADA MANUAL OF STANDARD PRACTICE AND CSA A23.3, UNLESS NOTED OTHERWISE.
- DO NOT ELIMINATE OR DISPLACE REINFORCING TO ACCOMMODATE HARDWARE. IF INSERTS CANNOT BE LOCATED AS SPECIFIED, OBTAIN APPROVAL OF ALL MODIFICATIONS FROM STRUCTURAL ENGINEER BEFORE PLACING CONCRETE.
- UNLESS INDICATED OTHERWISE, REINFORCING TO BE EXTENDED INTO ADJACENT CONCRETE ELEMENTS AND DEVELOPED WITH A STANDARD HOOK OR LAP SPLICE.
- PROVIDE DOWELS TO MATCH REINFORCING IN ALL PIERS, COLUMNS, WALLS AND CURBS. PROVIDE CLASS 'B' TENSION LAP SPLICE UNLESS NOTED OTHERWISE AND FULLY EMBED / DEVELOP REINFORCING.
- REINFORCING SHALL BE EFFECTIVELY CONTINUOUS AT ALL CORNERS AND INTERSECTIONS. HOOK AND SPLICE AS REQUIRED.
- STANDARD HOOKS SHALL BE USED UNLESS NOTED OTHERWISE.
- SPLICE REINFORCING AS INDICATED ON STRUCTURAL DRAWINGS OR OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER.
- BARs MARKED AS CONTINUOUS SHALL BE DEVELOPED BY CLASS 'B' TENSION LAP SPLICE IN ACCORDANCE WITH CSA A23.3.
- WHERE TENSION LAPS ARE SPECIFIED, PROVIDE MINIMUM CLASS 'B' TENSION LAP SPLICE IN ACCORDANCE WITH CSA A23.3.
- ALL SPLICES IN CORE AND SHEAR WALLS TO BE MINIMUM 1.5 x BASIC DEVELOPMENT LENGTH.
- MINIMUM WIRE MESH SPLICES SHALL BE 150mm.
- INSTALL ADHESIVE / EPOXY ANCHORS TO MANUFACTURER'S RECOMMENDATIONS. ALLOW TO REACH FULL DESIGN CAPACITY PRIOR TO LOADING.

CONCRETE COVER

- CONCRETE COVER TO REINFORCING BARS SHALL BE AS FOLLOWS OR AS NOTED ON THE DRAWINGS (WHICHEVER IS GREATER):

FOUNDATIONS - CAST AGAINST SOIL:	75 mm
FOUNDATIONS - NOT CAST AGAINST SOIL:	50 mm
WALLS:	40 mm
SLABS:	25 mm
BEAMS:	40 mm
COLUMNS:	40 mm
BALCONIES:	40 mm

(UN ON PLAN)	
(TO TIES)	
(TO TIES)	
(TO TOP STEEL)	
- IN AREAS REQUIRING OR SUPPORTING 2 - OR 3 - HOUR FIRE RESISTANCE RATINGS, ENSURE THAT CONCRETE COVER TO REINFORCING BARS ALSO SATISFIES THE FOLLOWING MINIMUM REQUIREMENTS:

SLAB BOTTOM STEEL	25mm
(ABOVE 2 HR FIRE RATED AREAS):	
SLAB BOTTOM STEEL	32mm
(ABOVE 3 HR FIRE RATED AREAS):	
COLUMNS / WALLS:	50mm
(TO TIES)	
- AT CLASS C-XL AND CLASS C-1 CONCRETE ELEMENTS, PROVIDE 60mm COVER TO BEAMS, SLABS AND WALLS.
- MAINTAIN SPECIFIED CONCRETE COVER AT ALL SLOPES, DEPRESSIONS, CORNERS AND CHANGES IN ELEVATION / THICKNESS.

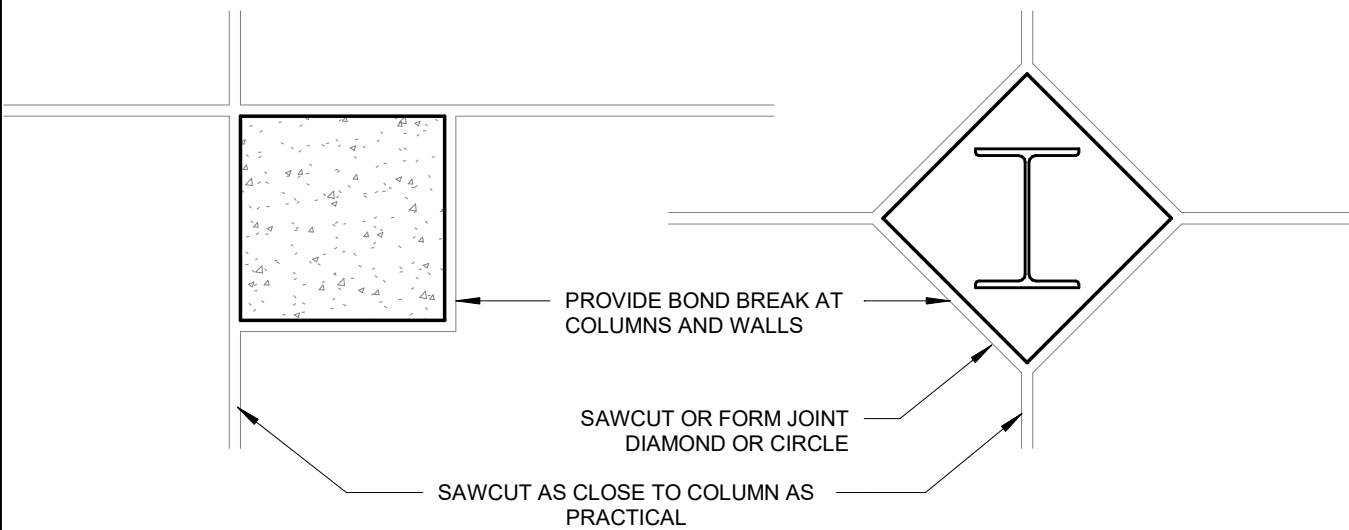
CONCRETE MIXES

- UNLESS NOTED OTHERWISE, PROPORTION NORMAL DENSITY CONCRETE IN ACCORDANCE WITH CSA A23.1 TO ACHIEVE THE FOLLOWING PERFORMANCE CHARACTERISTICS:

LOCATION	28-DAY STRENGTH	EXPOSURE CLASS	ENTRAINED AIR CONTENT
SLAB-ON-GRADE (EXTERIOR)	32 MPa	C-2	5 - 8%
SLAB-ON-GRADE (INTERIOR, PARKING)	25 MPa	C-4	-
SLAB-ON-GRADE (INTERIOR, NOT PARKING)	25 MPa	N / N-CF	-
FOOTINGS	25 MPa	N	-
FOUNDATION WALLS	25 MPa	F-2	4 - 7%
COLUMNS / PIERS / WALLS IN PARKING AREAS	35 MPa	C-1	5 - 8%
EXTERIOR COLUMNS / PIERS	25 MPa	F-2	4 - 7%
INTERIOR COLUMNS / PIERS	25 MPa	N	-
SHEAR & CORE WALLS	30 MPa	N	-
SUSPENDED SLABS	25 MPa	N	-
BALCONIES	35 MPa	F-2	4 - 7%
CONCRETE ON STEEL DECK	25 MPa	N	-
STAIRS AND LANDINGS	25 MPa	N	-
PAVEMENTS & SIDEWALKS	32 MPa	C-2	5 - 8%
RETAINING WALLS	35 MPa	C-1	5 - 8%
ALL OTHER WALLS	25 MPa	N	-
OTHER	25 MPa	N	-
- CONCRETE STRENGTHS NOTED ON SPECIFIC PLANS OR SCHEDULES TAKE PRECEDENCE OVER ABOVE VALUES.
- CONCRETE SHALL BE TYPE GU OR GU_h PORTLAND CEMENT UNLESS SPECIFIED OTHERWISE.
- IF BLENDED PORTLAND CEMENT / SLAG IS USED, SLAG CONTENT SHALL NOT BE MORE THAN 25% OF TOTAL MASS OF CEMENT.
- STAIRS AND LANDINGS IN PARKING AREAS SHALL HAVE CORROSION INHIBITOR INCORPORATED IN CONCRETE MIX.
- USE OF CALCIUM CHLORIDE IS NOT PERMITTED.
- NON-SHRINK CEMENTITIOUS GROUT SHALL BE AN APPROVED PRE-MIXED PROPRIETARY PRODUCT. COMPRESSIVE STRENGTH AT 28-DAYS = 50 MPa.
- REFER TO STRUCTURAL AND ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS FOR EXPOSED CONCRETE FINISHES.
- SUBMIT CONCRETE MIX DESIGNS FOR REVIEW.

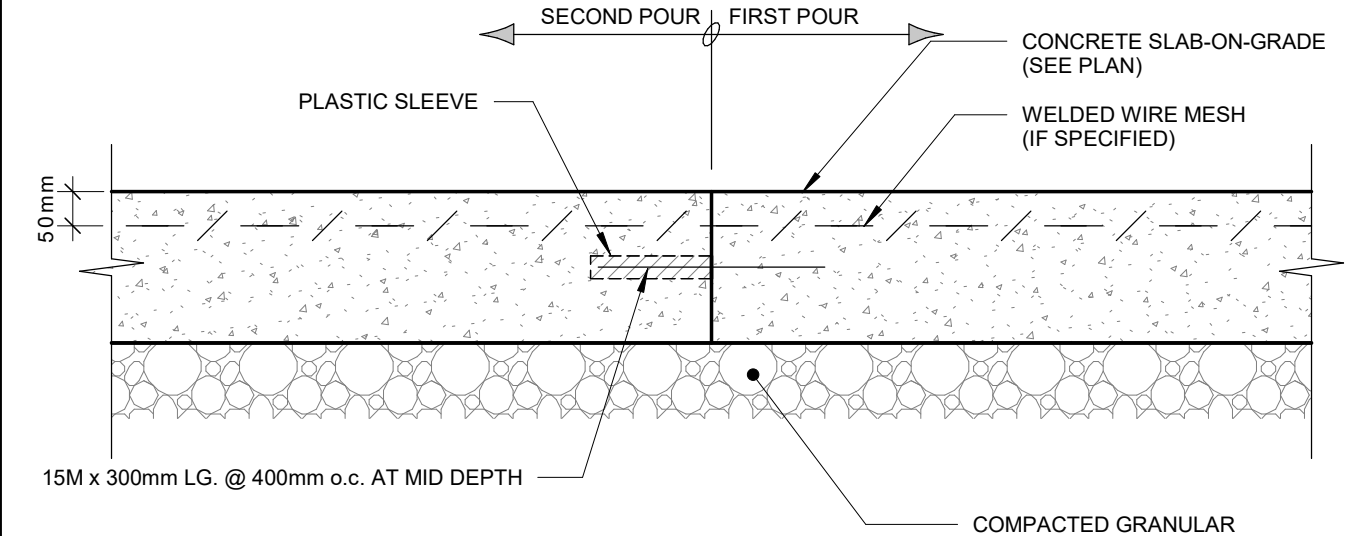
SLAB-ON-GRADE

- SLAB-ON-GRADE TO BE PLACED ON COMPACTED GRANULAR MATERIAL IN ACCORDANCE WITH THE GEOTECHNICAL REPORT. COMPACTION TESTS ON FILL MATERIAL TO BE CARRIED OUT PRIOR TO SLAB-ON-GRADE PLACEMENT.
- PROVIDE SAWCUTS AT MAX. 25 TIMES THE SLAB THICKNESS AND NOT MORE THAN 4.5m SPACING. CUT 1/3 DEPTH OF SLAB AND FILL WITH FLEXIBLE JOINT FILLER.
- SAWCUT SLAB-ON-GRADE AS SOON AS CONCRETE HAS SUFFICIENTLY SET TO AVOID RAVELLING THE EDGE (APPROX. 8-24 HOURS AFTER CONCRETE PLACEMENT).
- PROVIDE BOND BREAK BETWEEN SLAB AND VERTICAL SURFACES USING ASPHALT IMPREGNATED FIBREBOARD OR HEAVY DUTY POLYETHYLENE.
- DO NOT PLACE SLAB-ON-GRADE IN ONE CONTINUOUS POUR IN LENGTHS EXCEEDING 30m EITHER DIRECTION, UNLESS OTHERWISE NOTED ON PLAN.
- CONTRACTOR TO SUBMIT PROPOSED SAWCUT PATTERNS / LOCATIONS TO ENGINEER FOR REVIEW. SAWCUTS TO BREAK AT COLUMNS AND CORNERS AND BE AS SQUARE AS POSSIBLE.
- MAINTAIN MINIMUM SPECIFIED THICKNESS AT ALL DEPRESSIONS AND CHANGES IN ELEVATIONS. REFER TO ARCHITECTURAL DRAWINGS FOR ALL EXTENTS AND LOCATIONS OF FINISHES AND DEPRESSIONS.
- WHERE CONCRETE FIBRES ARE SPECIFIED FOR SLAB-ON-GRADE REINFORCING, MIN. DOSAGE RATE TO BE 1.8 kg/m³ OF TUF-STRAND SF MACRO SYNTHETIC FIBRES BY EUCLID CHEMICALS - UNLESS NOTED OTHERWISE.
- WHERE WIRE MESH IS SPECIFIED FOR SLAB-ON-GRADE REINFORCING, MIN. SIZE OF MESH TO BE 152x152 MW 18.7 x 18.7.

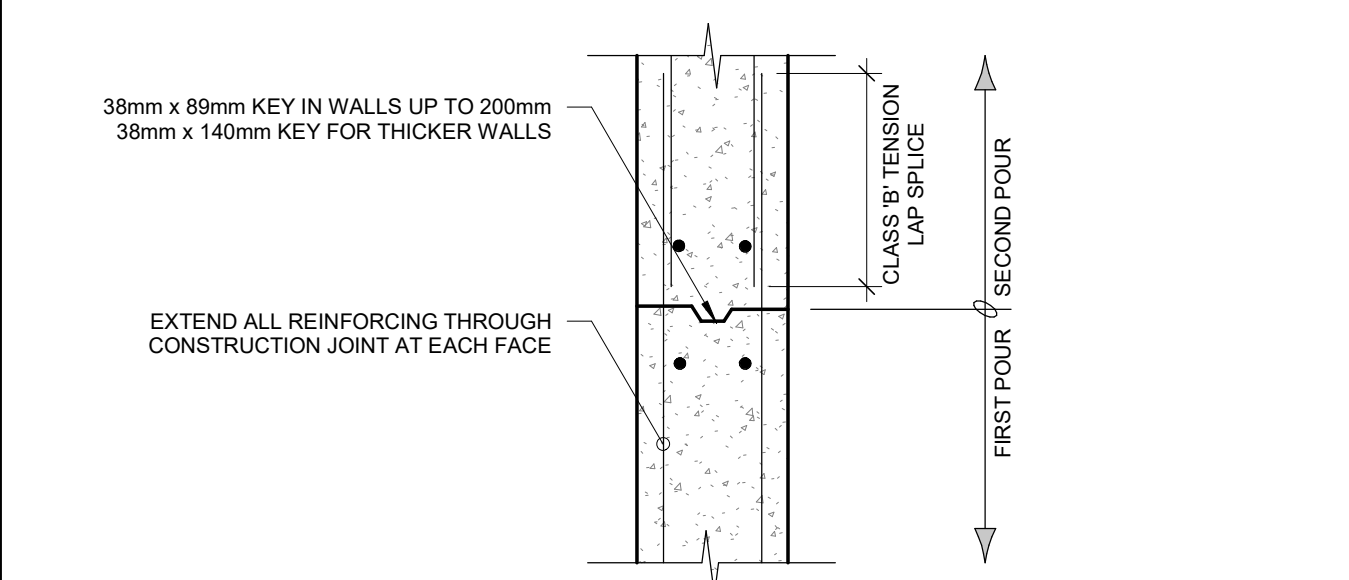


CONSTRUCTION JOINTS

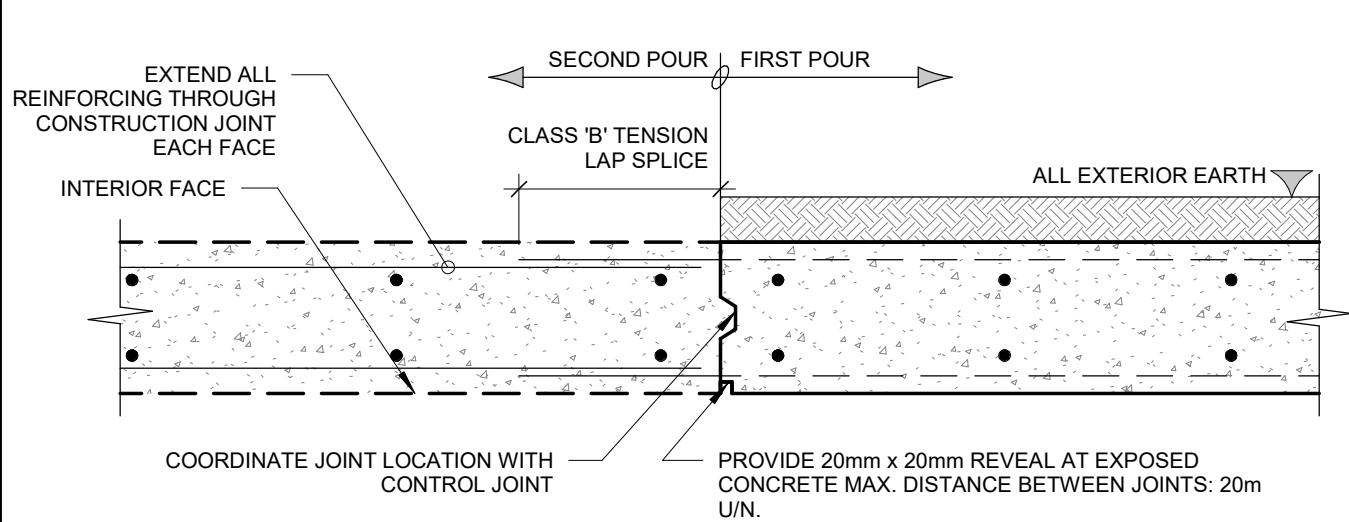
CONSTRUCTION JOINT IN SLAB-ON-GRADE



HORIZONTAL CONSTRUCTION JOINT



VERTICAL CONSTRUCTION JOINT



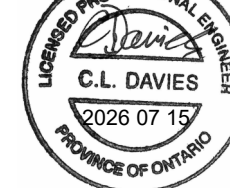
NOTES:

- PROVIDE CONSTRUCTION JOINTS AS INDICATED ON STRUCTURAL DRAWINGS.
- CONTRACTOR SHALL SUBMIT DETAILS OF PROPOSED CONSTRUCTION JOINT LOCATIONS NOT INDICATED ON DRAWINGS TO THE STRUCTURAL ENGINEER FOR REVIEW.
- CONTRACTOR TO SUBMIT PROPOSED CONSTRUCTION JOINT LOCATIONS TO ENGINEER FOR REVIEW PRIOR TO POURING.

#	DATE	DESCRIPTION
3	2026 07 15	ISSUED FOR PERMIT / TENDER
2	2026 06 19	ISSUED FOR FINAL COORDINATION
1	2026 06 05	ISSUED FOR COORDINATION

STAMP:

NORTH ARROW:



PROJECT NAME AND ADDRESS:
TOWN OF SMITH FALLS
PUBLIC WORKS
OPERATIONS CENTRE

11 AIR SPACE DR.
SMITHS FALLS, ON, K7A 0B2

DRAWING NAME:
GENERAL NOTES AND
DETAILS

DESIGNED BY: S. HURD

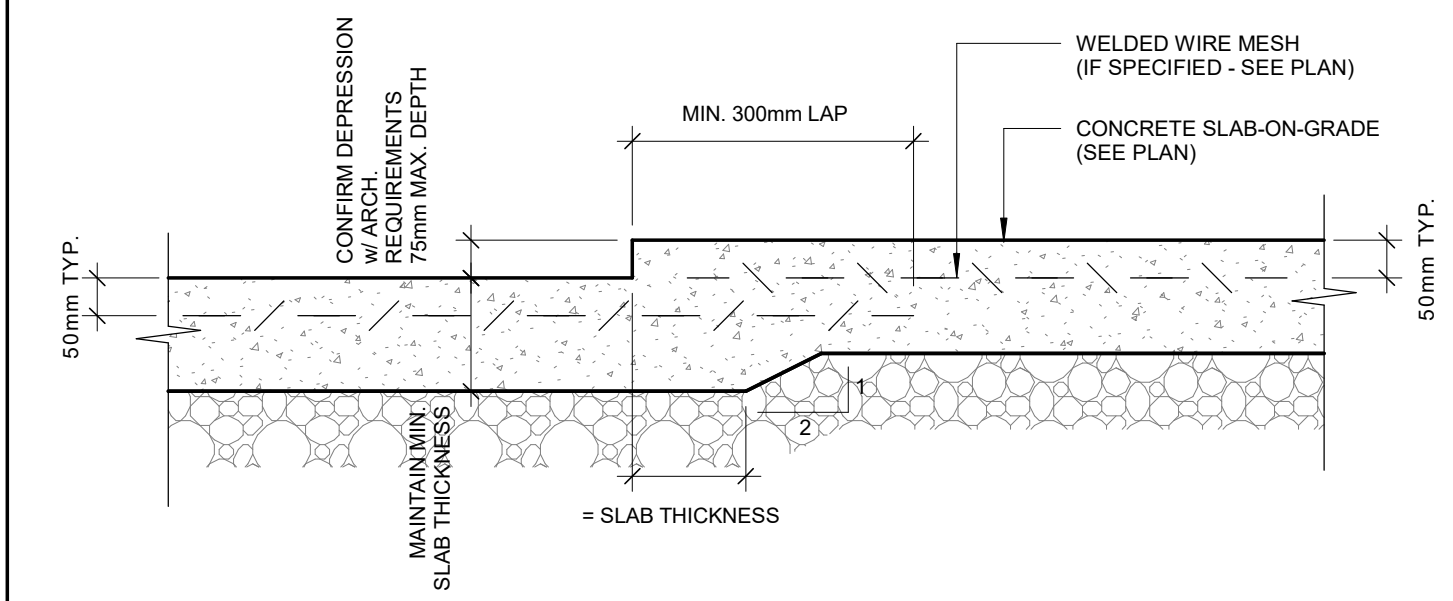
DRAWN BY: J. LABBE

START DATE: 2026 05 29

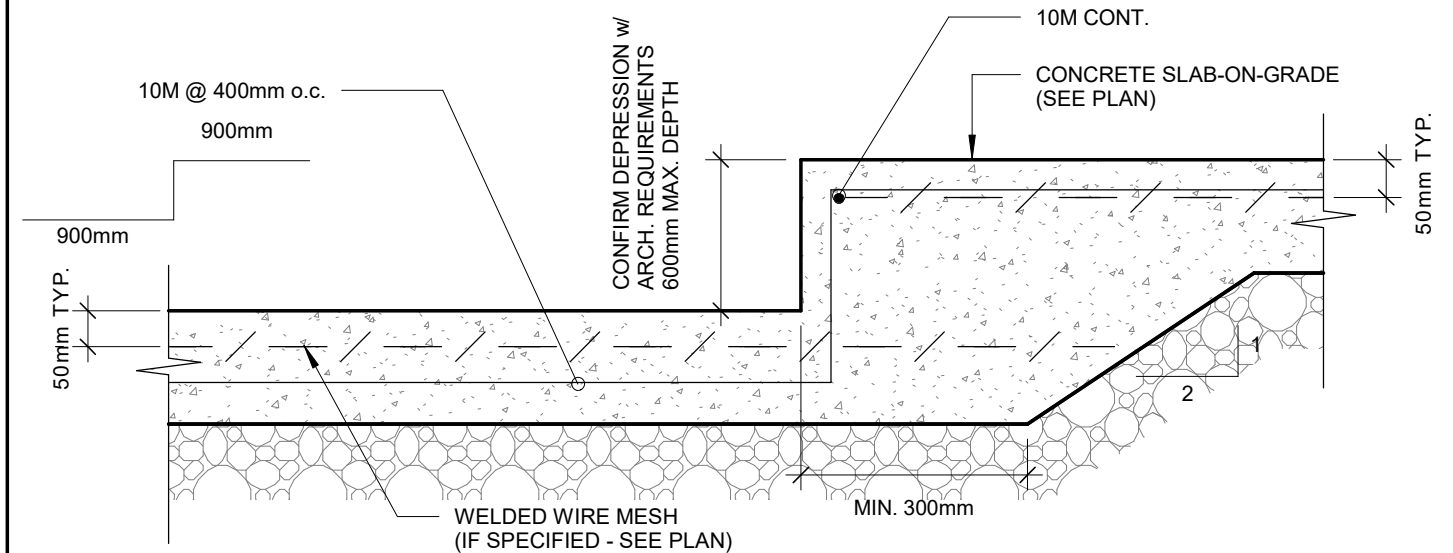
PROJECT #: 26-097

S-002

DEPRESSION IN SLAB-ON-GRADE ($\leq 75\text{mm}$)

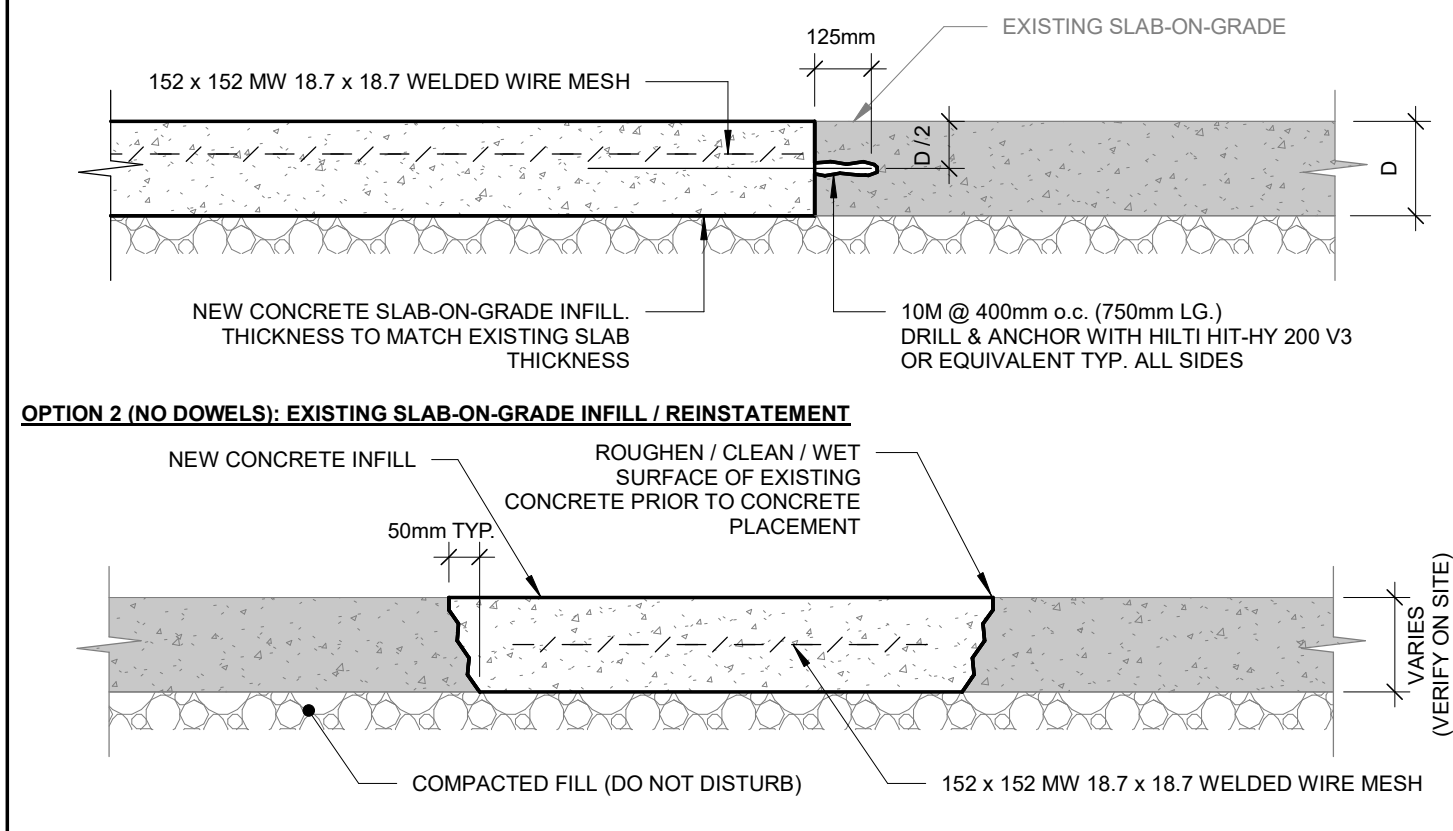


STEP IN SLAB-ON-GRADE (> 75mm, MAX. 600mm)



EXISTING SLAB INFILLS

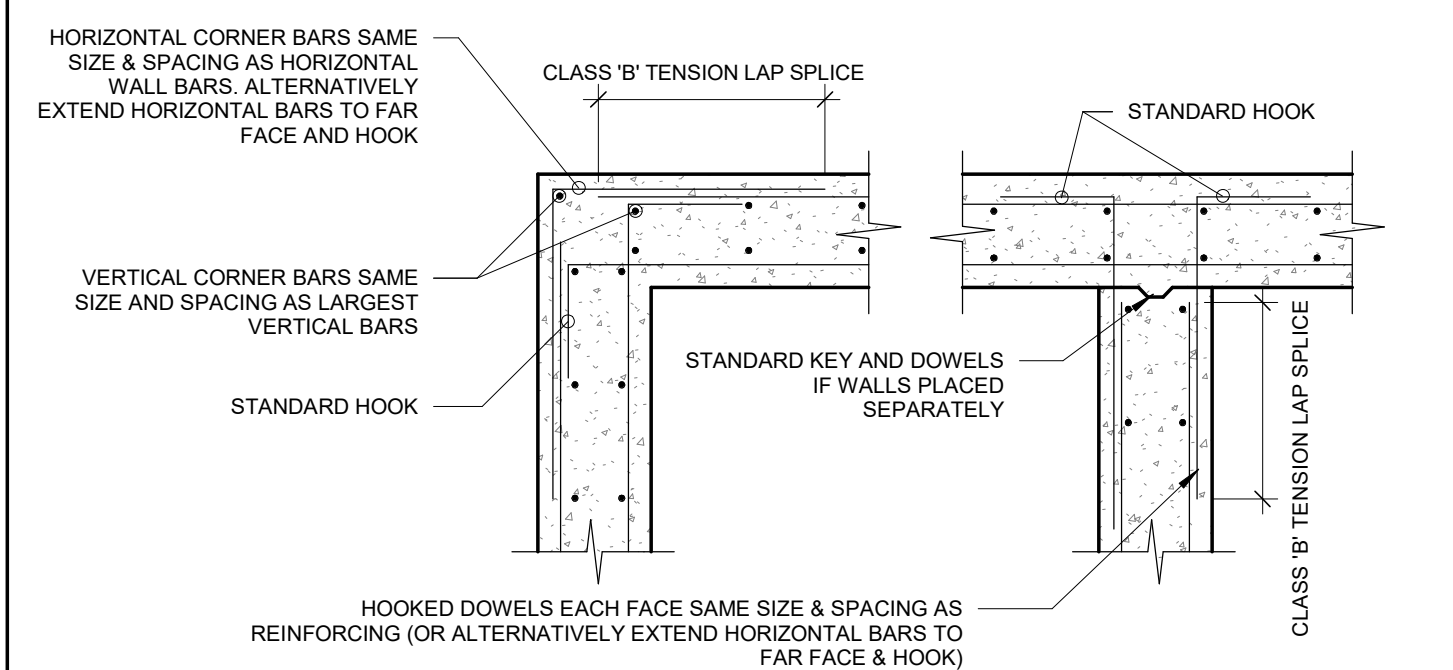
OPTION 1 (DOWELS): EXISTING SLAB-ON-GRADE INFILL / REINSTATEMENT



SLAB AND WALL OPENINGS

1. DISPLACE BARS LATERALLY AT SLAB OPENINGS. **DO NOT CUT**. PLACE HALF OF DISPLACED BARS EACH SIDE OF OPENING AND INFILL BETWEEN WITH BARS AS NOTED ON REINFORCING PLAN.
2. PROVIDE 1-15M TOP AND BOTTOM MINIMUM ADDITIONAL REINFORCING AROUND SLAB OPENINGS 300mm x 300mm OR LARGER (UNLESS NOTED). EXTEND BAR MIN. CLASS 'B' TENSION LAP SPLICE BEYOND FACE OF OPENING.
3. PROVIDE THE FOLLOWING MINIMUM ADDITIONAL REINFORCING AROUND WALL OPENINGS 300mm x 300mm OR LARGER (UNLESS NOTED). EXTEND BAR MIN. CLASS 'B' TENSION LAP SPLICE BEYOND FACE OF OPENING EACH WAY.
 - 200mm WALLS: 1-20M
 - 250mm WALLS: 2-20M E.F.
 - 300mm OR THICKER WALLS: 2-20M E.F.
4. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR ADDITIONAL OPENINGS TO THOSE INDICATED.
5. COORDINATE INSTALLATION OF OPENINGS, SLEEVES, CONDUITS AND OTHER CAST-IN INSERTS WITH SIZES AND LOCATIONS SHOWN ON ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS. OBTAIN APPROVAL OF STRUCTURAL ENGINEER FOR ANY OPENINGS, SLEEVES, CONDUITS, OR INSERTS NOT SHOWN ON STRUCTURAL DRAWINGS.
 ALL OPENINGS MUST BE FORMED BEFORE THE SLAB OR WALL IS POURED. DO NOT CUT ANY OPENINGS AFTER CONCRETING.
 - UNLESS SPECIFICALLY AUTHORIZED BY THE ENGINEER.
 - ENGINEER'S APPROVAL MUST BE OBTAINED FOR ANY CONCENTRATION OF SLEEVES IN COLUMN BAND AND AROUND COLUMN.
6. PROVIDE ADDITIONAL REINFORCING AROUND SLAB OPENINGS AS INDICATED ON SLAB REINFORCING PLANS

HORIZONTAL WALL REINFORCING AT CORNERS / INTERSECTIONS



GENERAL

GENERAL

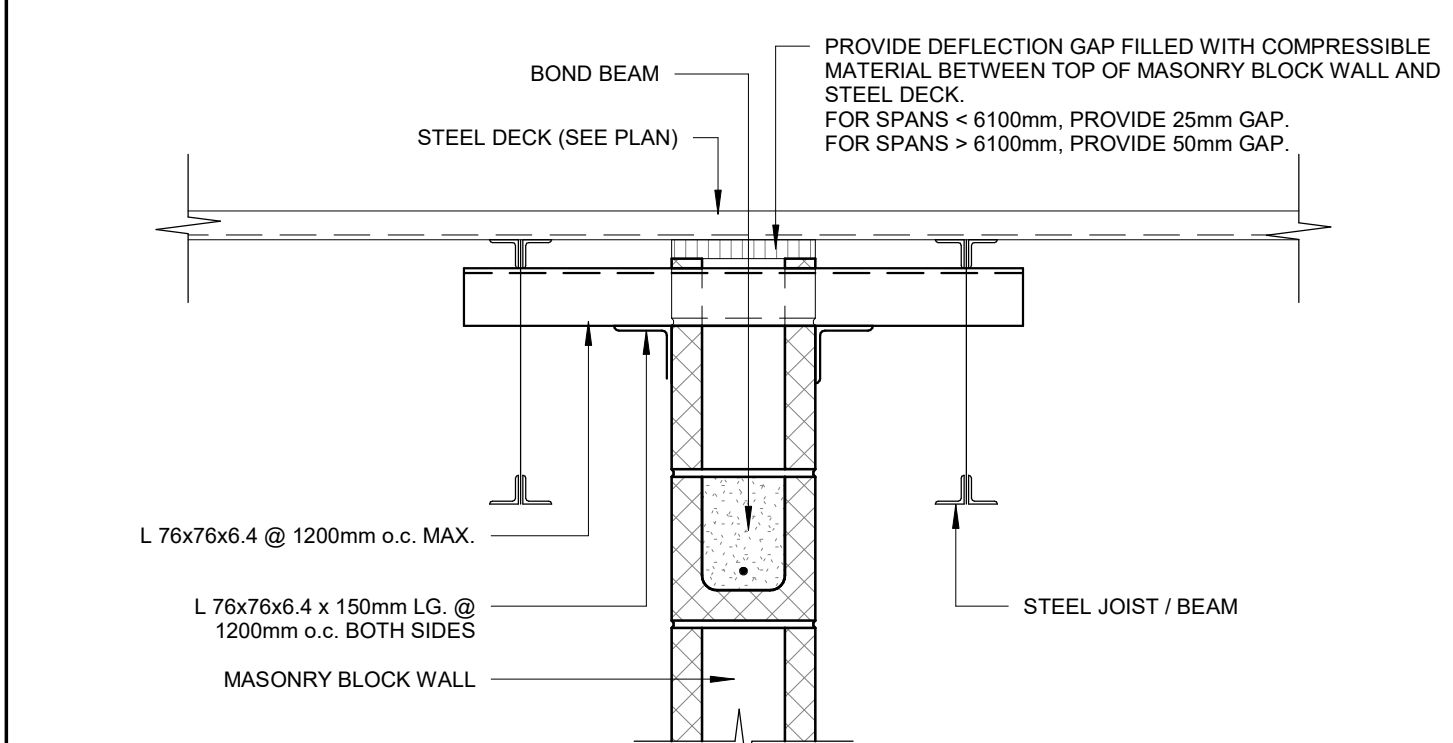
1. ALL MASONRY WORK SHALL CONFORM TO THE ONTARIO BUILDING CODE AND CSA S304 (DESIGN OF MASONRY STRUCTURES) AND CSA A371 (MASONRY CONSTRUCTION FOR BUILDINGS).
2. ALL CONCRETE MASONRY UNITS SHALL CONFORM TO CSA A165 SERIES AND AS FOLLOWS:
CLASSIFICATION: H / 15 / A / M
3. GROUT SHALL CONFORM TO CSA A179 AND AS FOLLOWS:
TYPE: COARSE GROUT, NON-SHRINK
MINIMUM STRENGTH: 15 MPa AT 28 DAYS
SLUMP: 200mm - 250mm
4. FILL ALL CELLS CONTAINING ANCHORS OR REINFORCING WITH GROUT.
5. MORTAR SHALL CONFORM TO CSA A179, TYPE S (MIN. STRENGTH = 12.4 MPa).
6. REINFORCE MASONRY AS INDICATED.
7. VERTICAL REINFORCING SHALL BE PLACED IN THE CENTRE OF GROUTED CORES.
8. VERTICAL AND BOND BEAM REINFORCING SHALL BE LAPPED AND HOOKED. PROVIDE CLASS 'B' TENSION LAP SPLICE UNLESS NOTED OTHERWISE AND FULLY EMBED / DEVELOP REINFORCING.
9. MINIMUM REINFORCING (UNLESS NOTED OTHERWISE ON STRUCTURAL DRAWINGS):

THICKNESS	LOAD-BEARING WALLS	NON LOAD-BEARING WALLS
140mm	SMR EVERY COURSE 15M @ 800mm o.c. VERT.	SMR ALT. COURSES 10M @ 800mm o.c. VERT.
190mm	HDMR EVERY COURSE 15M @ 800mm o.c. VERT.	HDMR ALT. COURSES 15M @ 1200mm o.c. VERT.
240mm	HDMR EVERY COURSE 15M @ 800mm o.c. VERT.	HDMR ALT. COURSES 15M @ 1000mm o.c. VERT.
290mm	HDMR EVERY COURSE 20M @ 800mm o.c. VERT.	HDMR ALT. COURSES 15M @ 800mm o.c. VERT.
10. PROVIDE ADDITIONAL 15M VERT. IN GROUT FILLED CORES EACH SIDE OF EVERY OPENING EXCEEDING 1000mm IN ANY DIMENSION AND AT EVERY CORNER, END OF WALL, OR OTHER DISCONTINUITY.
11. FIRST TWO COURSES OF MASONRY SHOULD BE GROUTED SOLID.
12. PROVIDE FULLY GROUTED BOND BEAMS AT THE TOP OF EVERY WALL AND AT EVERY FLOOR:
NON-LOAD BEARING WALLS: MIN. 1 COURSE DEEP, REINFORCED WITH MIN. 1-20M
LOAD BEARING WALLS: MIN. 2 COURSES DEEP, REINFORCED WITH MIN. 1-20M
13. PROVIDE LATERAL SUPPORTS (DEFLECTION CLIPS) AT THE TOP OF ALL MASONRY WALLS AS INDICATED IN TYPICAL DETAILS OR UNLESS NOTED OTHERWISE ON STRUCTURAL DRAWINGS.
14. PROVIDE UNTELS OVER ALL OPENINGS OR RECESSES IN MASONRY WALLS EXCEEDING 400mm IN WIDTH, AS INDICATED IN TYPICAL DETAILS OR ON STRUCTURAL DRAWINGS.
15. CENTER BEARING PLATES OR WALL PLATES ON BEAM CENTERLINE AND CENTERLINE OF WALL (UNLESS INDICATED OTHERWISE).
16. PLACE BEARING PLATES AND SHELF ANGLES BACK MINIMUM 12mm FROM FACE OF WALL.
17. PROVIDE +/- 20mm GROUT UNDER ALL BEARING PLATES.

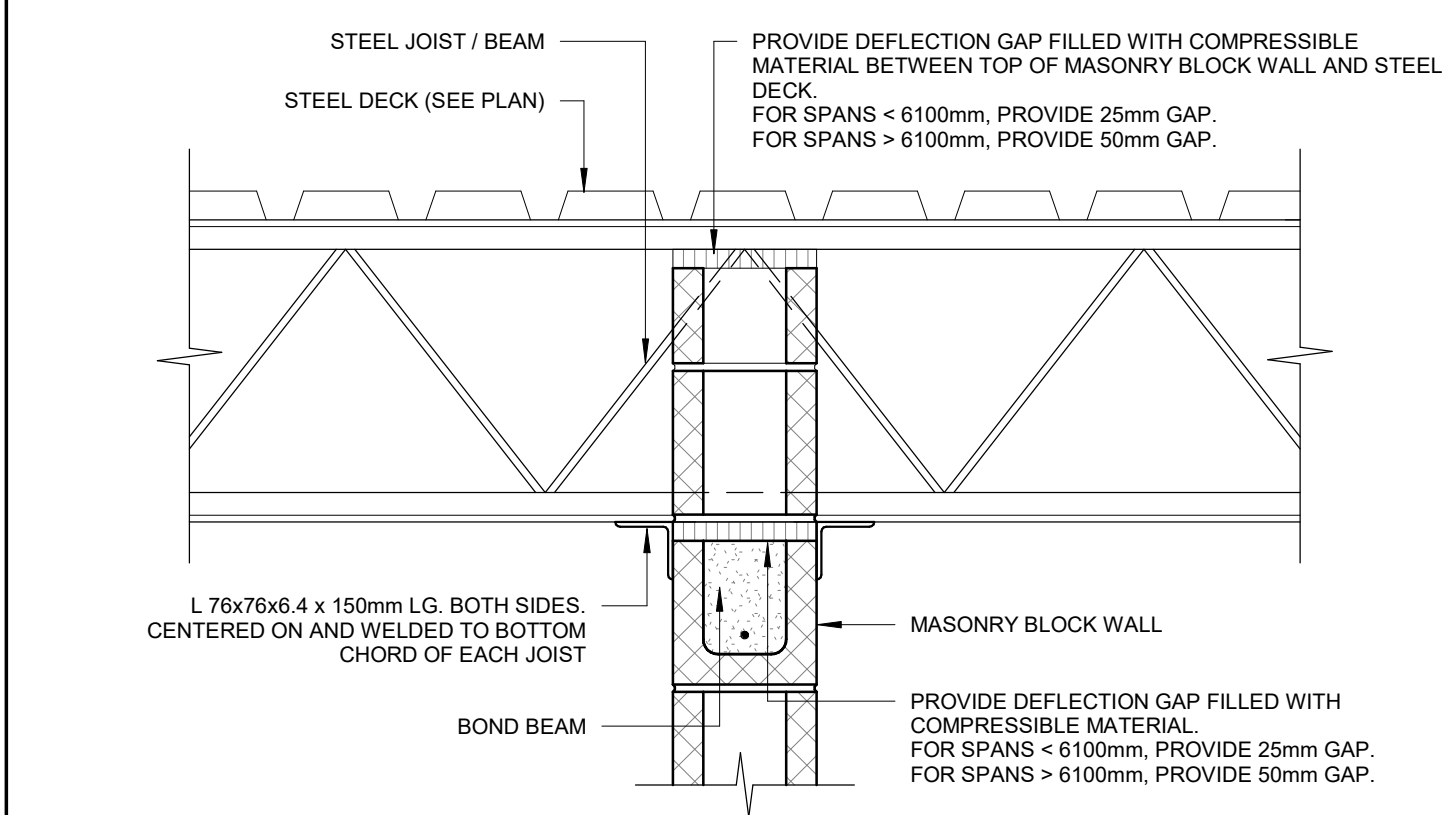
THICKNESS	LOAD-BEARING WALLS	NON LOAD-BEARING WALLS
140mm	SMR EVERY COURSE 15M @ 600mm o.c. VERT.	SMR ALT. COURSES 10M @ 600mm o.c. VERT.
190mm	HDIMR EVERY COURSE 15M @ 800mm o.c. VERT.	HDIMR ALT. COURSES 15M @ 1200mm o.c. VERT.
240mm	HDIMR EVERY COURSE 15M @ 600mm o.c. VERT.	HDIMR ALT. COURSES 15M @ 1000mm o.c. VERT.
290mm	HDIMR EVERY COURSE 20M @ 600mm o.c. VERT.	HDIMR ALT. COURSES 15M @ 800mm o.c. VERT.

DEFLECTION CLIPS (NON-LOAD BEARING MASONRY WALL)

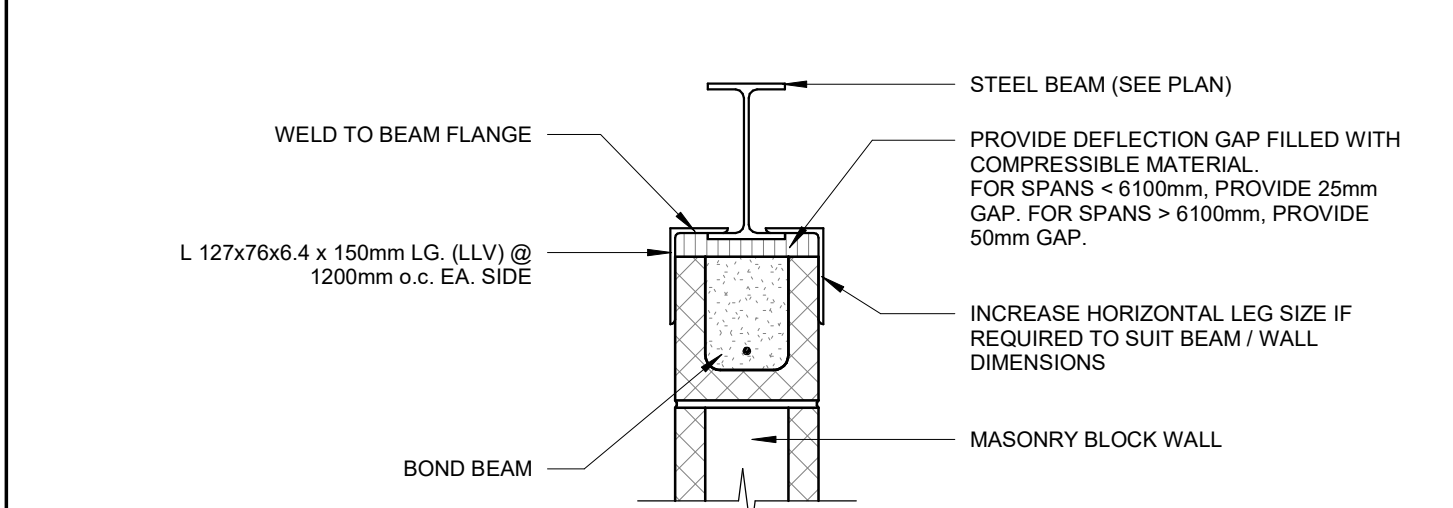
AT STEEL STRUCTURE - WALL PARALLEL TO JOIST / BEAM



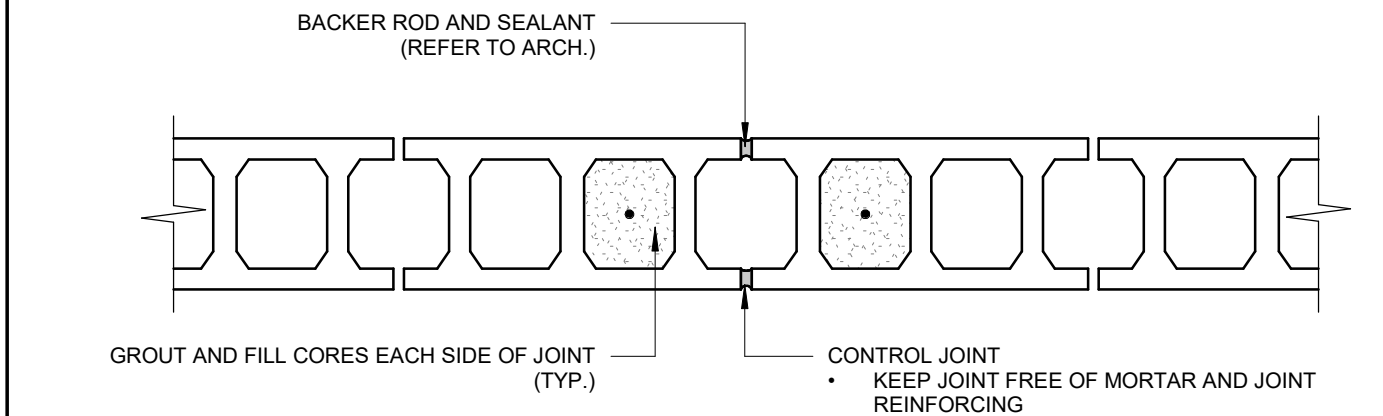
AT STEEL STRUCTURE - WALL PERPENDICULAR OR ANGLED TO JOIST / BEAM



MASONRY BLOCK WALL UNDER STEEL BEAM (NON-LOAD BEARING MASONRY WALL)



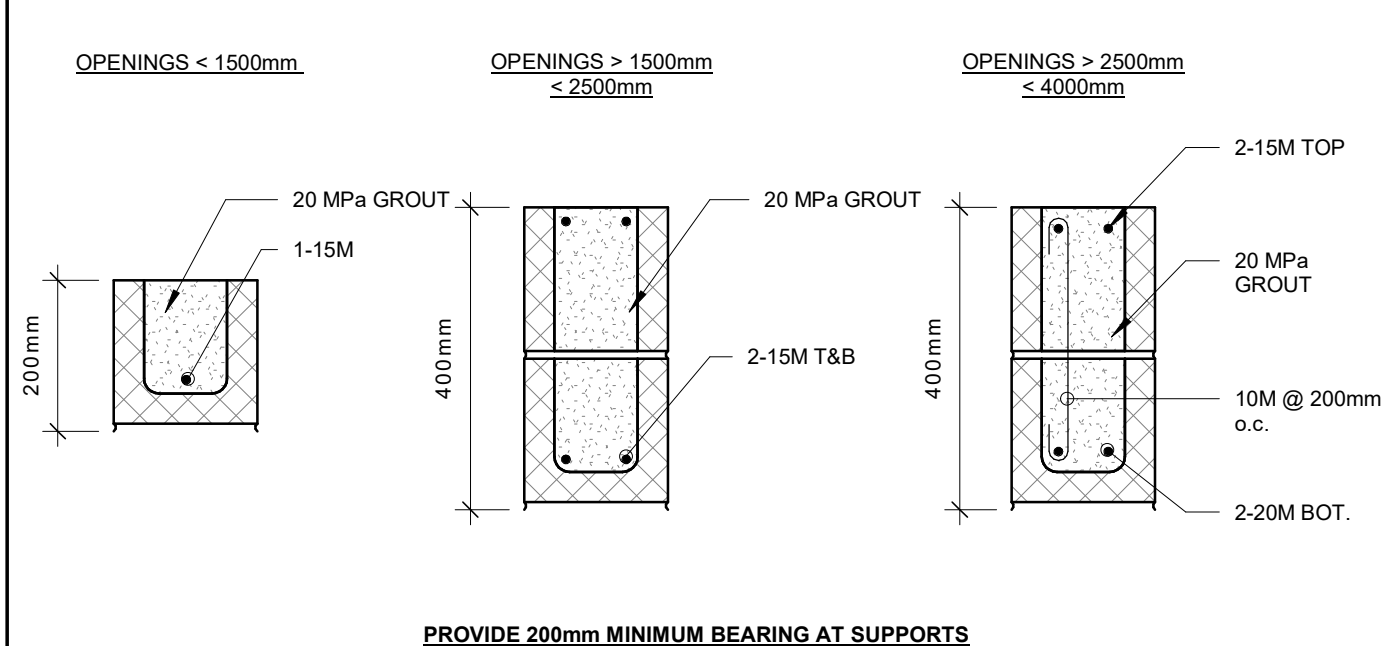
MOVEMENT JOINTS IN MASONRY BLOCK WALL



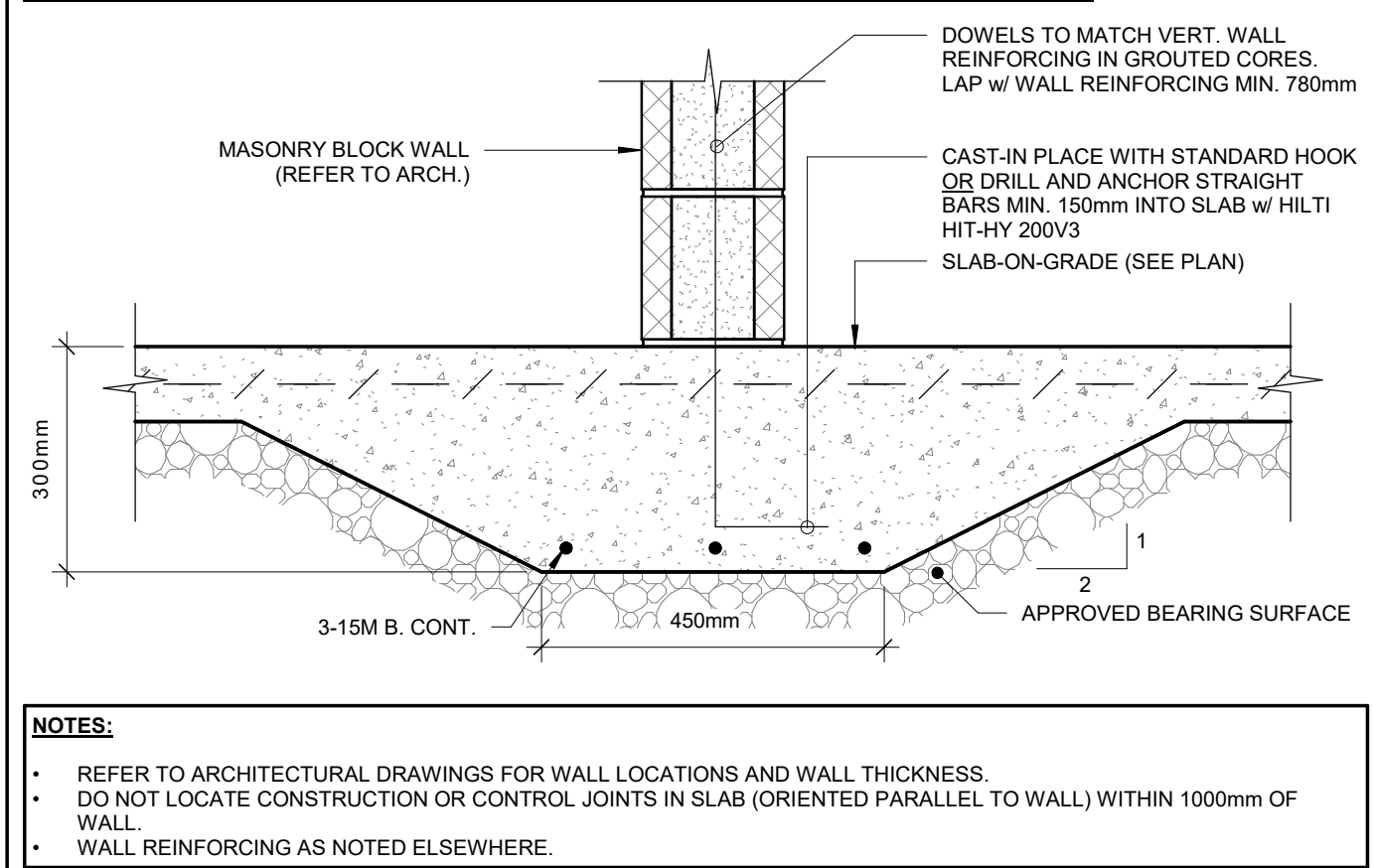
NOTES:

- BOND BEAM AT TOP OF WALL TO BE CONTINUOUS (NO MOVEMENT JOINTS)
- PROVIDE MOVEMENT JOINT AT 7.6m SPACING MAX.

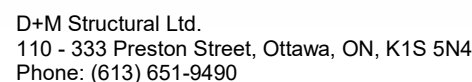
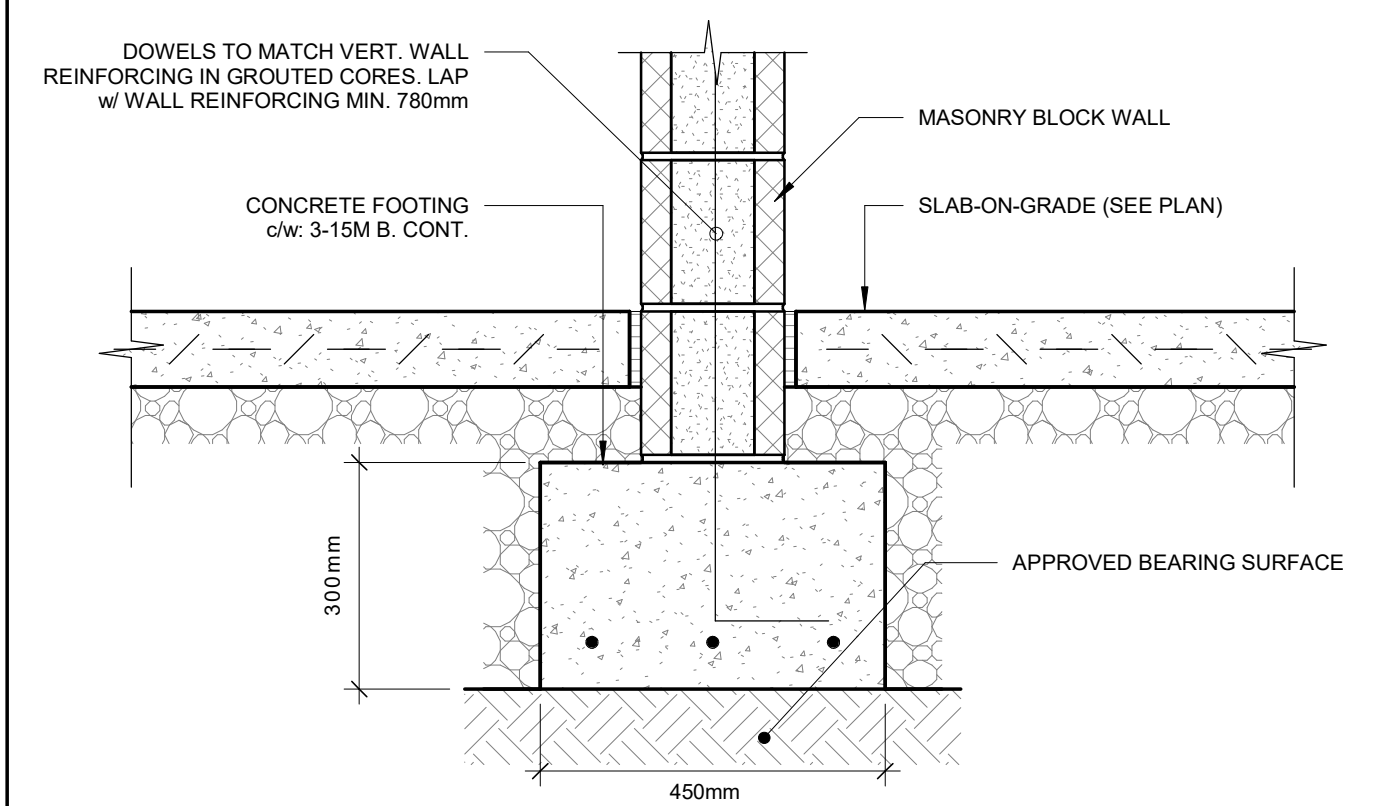
MASONRY LINTELS (NON-LOAD BEARING MASONRY WALLS)



SLAB THICKENING AT NON-LOAD BEARING MASONRY BLOCK WALL



FOUNDATION AT NON-LOAD BEARING MASONRY BLOCK WALL

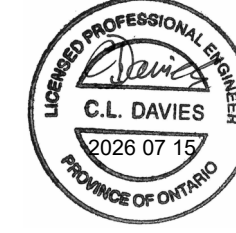


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1	2026 06 05	ISSUED FOR COORDINATION
#	DATE	DESCRIPTION

STAMP:

NORTH ARROW:



PROJECT NAME AND ADDRESS:
TOWN OF SMITH FALLS
PUBLIC WORKS
OPERATIONS CENTRE

11 AIR SPACE DR.
SMITHS FALLS, ON. K7A 0B2

DRAWING NAME:

GENERAL NOTES AND
DETAILS

DESIGNED BY: S. HURD

DRAWN BY: J. LABBÉ

START DATE: 2026 05 29

PROJECT #: 26-09

S-003

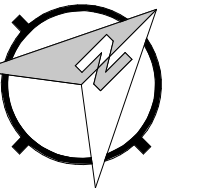
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PROJECT NAME AND ADDRESS:
TOWN OF SMITH FALLS
PUBLIC WORKS
OPERATIONS CENTRE

AIR SPACE DR.
SMITHS FALLS, ON. K7A 0B2

DRAWING NAME:
MAIN BUILDING
FOUNDATION / GROUND
FLOOR PLAN

DESIGNED BY: S. HURD

RAWN BY: J. LABBÉ

START DATE: 2026 05 29

PROJECT #: 26-097

S-100

200mm CONCRETE PAD. SEE SECTION FOR REINFORCING DETAILS

SEE SECTION FOR RIGID INSULATION REQUIREMENTS

1500 mm

1500 mm

1500 mm

1500 mm

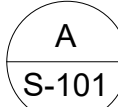
GENERATOR

GARBAGE ENCLOSURE

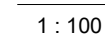
A

S-101

1 : 100



SCALE: 1 : 25



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PROJECT #: 26-09

ROOF DECK

38mm x 0.91mm GALV. STEEL ROOF DECK (TYP. U/N). c/w:

- HILTI X-HSN 24 OR HILTI X-ENP-19 FASTENERS AS SPECIFIED BELOW
- 36/4 PATTERN (EVERY OTHER FLUTE)
- SIDE LAP WITH HILTI S-SLC 01 M HWH SIDE LAP CONNECTOR @ 300mm o.c.

NOTES

- WHERE STEEL THICKNESS IS BETWEEN 3.2mm AND 9.5mm, HILTI X-HSN 24 FASTENERS ARE SUITABLE.
- WHERE STEEL THICKNESS IS GREATER THAN 6.4mm, HILTI X-ENP-19 FASTENERS ARE SUITABLE

1. T.J. DENOTES TIE JOIST.

2. 102mm DP, OWSJ SHOES (TYP. UN).
3. EXTEND JOIST SHOE TO ROOF DECK EDGE AS REQUIRED.
4. ALL JOISTS ARE TO CAMBERED FOR FULL DEAD LOAD DEFLECTION.
5. DESIGN OWSJ SHOES FOR THE FOLLOWING ROLL-OVER FORCES (PLUS APPLICABLE AMPLIFICATION FACTORS) DUE TO SEISMIC LOADS:
 - * GL 1 = 10 kN
 - * GL 3 = 10 kN

[illegible]

WT DENOTES OPERATING WEIGHT OF ROOFTOP UNIT (kN)

100 kN DENOTES SEISMIC AXIAL LOAD IN DIAPHRAGM COLLECTOR BEAMS (FOR CONNECTION DESIGN). LOADS ARE UNAMPLIFIED.

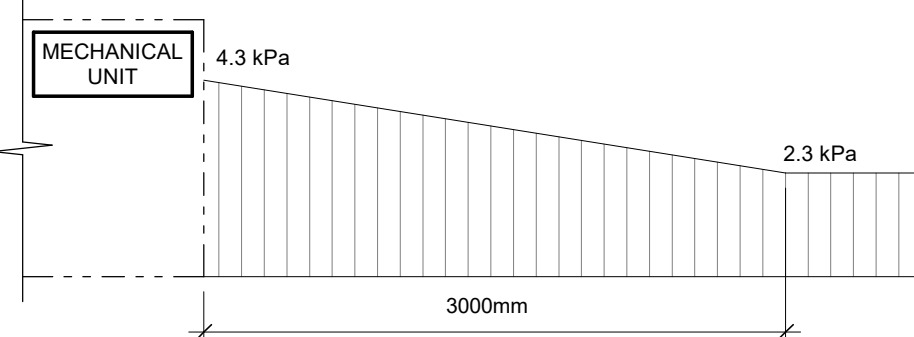
► DENOTES MOMENT / SHEAR CONNECTION. SEE PLANS FOR LOCATIONS AND DESIGN LOAD

R= 75 kN DENOTES FACTORED REACTION (ULS) AT END OF STEEL BEAM (FOR CONNECTION DESIGN)
LOADS ARE FACTORED (ULS) U/N.

 DENOTES TOP OF STEEL ELEVATION (i.e. UNDERSIDE OF DECK) AT END OF BEAM / OWSJ
RELATIVE TO HIGH POINT (REFER TO ARCH. FOR ADD'L INFO). BEAMS SUPPORTING OWSJ TO
BE LOWERED BY OWSJ SHOE DEPTH.

- SEE ELEVATIONS ON S-300 SERIES FOR CONFIGURATION / DESIGN LOADS IN BRACED FRAMES
MOMENT FRAMES / SHEAR WALLS.
- PROVIDE ADDITIONAL FRAMING UNDER ROOFTOP UNIT CURBS AND AROUND OPENINGS (SEE
GENERAL NOTES).

- UP TO 2.0m ABOVE ROOF, MAX. 4.5m LONG.
- CONTRACTOR TO CONFIRM UNIT / CURB HEIGHT PRIOR TO FABRICATING JOISTS.



100 kN

100 kN DENOTES SEISMIC AXIAL LOAD IN DIAPHRAGM COLLECTOR BEAMS (FOR CONNECTION DESIGN). LOADS HAVE NOT BEEN AMPLIFIED BY R_d .

— DENOTES MOMENT / SHEAR CONNECTION. SEE PLANS FOR LOCATIONS AND DESIGN LOAD

R= 75 kN DENOTES FACTORED REACTION (ULS) AT END OF STEEL BEAM (FOR CONNECTION DESIGN)
LOADS ARE FACTORED (ULS) U/N.

- SEE ELEVATIONS ON S-300 SERIES FOR CONFIGURATION / DESIGN LOADS IN BRACED FRAMES / MOMENT FRAMES / SHEAR WALLS.
- PROVIDE ADDITIONAL FRAMING UNDER MECHANICAL UNIT CURBS AND AROUND OPENINGS (SEE GENERAL NOTES AND DETAILS).

1. T.J. DENOTES TIE JOIST

2. 102mm DP, OWSJ SHOES (TYP. UN).
3. EXTEND JOIST SHOE TO SLAB EDGE AS REQUIRED.
4. ALL JOISTS ARE TO BE CAMBERED FOR FULL DEAD LOAD DEFLECTION.
5. DESIGN OWSJ SHOES FOR THE FOLLOWING ROLLVER FORCES (PLUS APPLICABLE AMPLIFICATION FACTORS) DUE TO SEISMIC LOADS:
 - GRID 9 = 10 kN
 - GRID 10 = 10 kN

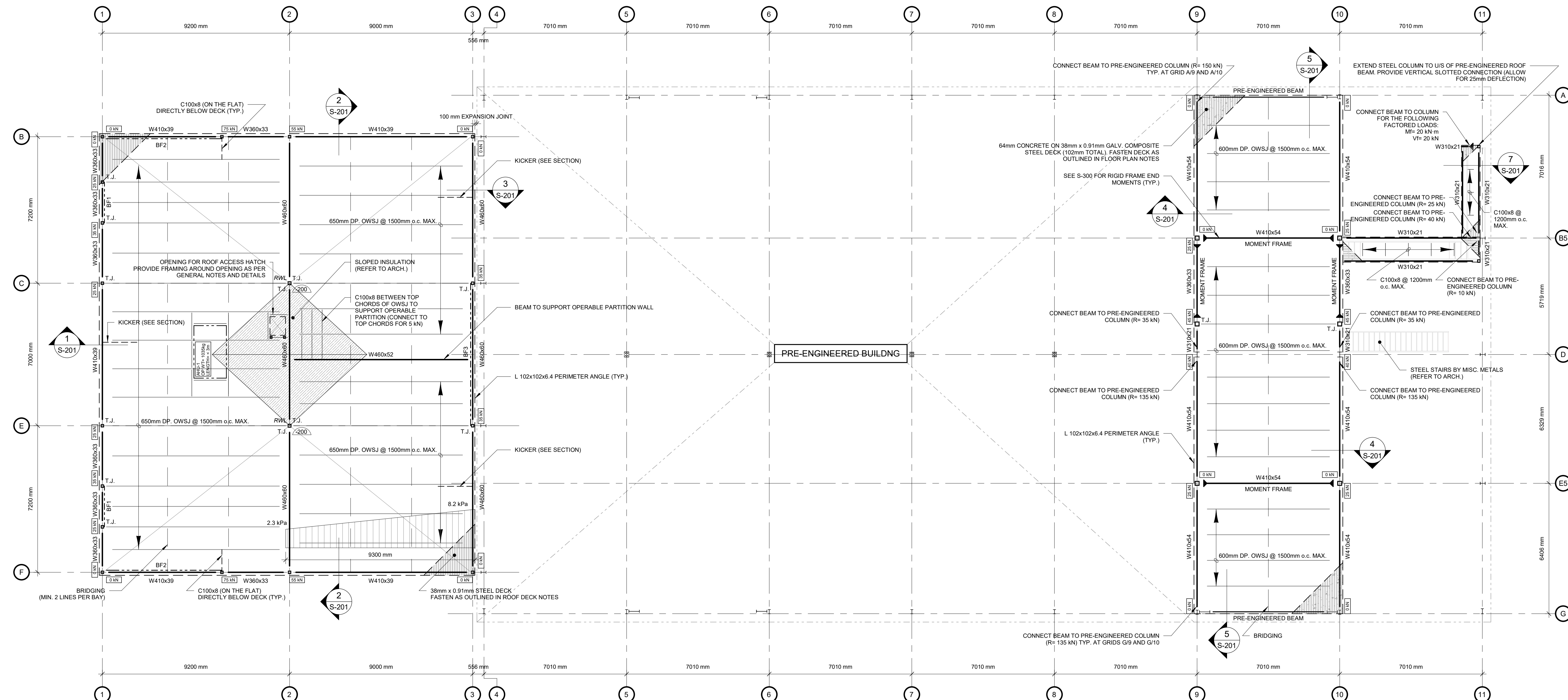
COMPOSITE SLAB:

64mm CONCRETE ON 38mm x 0.91mm GALV. COMPOSITE STEEL DECK (102mm TOTAL). REINFORCE WITH 152x152 WM 18.7x18.7 WIRE MESH c/c:

- HILTI X-HSN 24 OR HILTI X-ENP-19 FASTENERS AS SPECIFIED BELOW
- 36/4 PATTERN (ALTERNATE FLUTES)
- SIDE LAP WITH HILTI S-SLC 01 M HWH SIDE LAP CONNECTOR @ 600mm o.c.

NOTES:

- WHERE STEEL THICKNESS IS BETWEEN 3.2mm AND 9.5mm, HILTI X-HSN 24 FASTENERS ARE SUITABLE.
- WHERE STEEL THICKNESS IS GREATER THAN 6.4mm, HILTI X-ENP-19 FASTENERS ARE SUITABLE



MAIN BUILDING MEZZANINE / OFFICE BUILDING ROOF PLAN

1 : 100



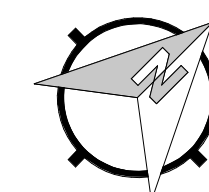
D+M Structural Ltd.
110 - 333 Preston Street, Ottawa, ON, K1S 5N4
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TOWN OF SMITH FALLS
PUBLIC WORKS
OPERATIONS CENTRE

11 AIR SPACE DR.
SMITHS FALLS, ON. K7A 0B2

DRAWING NAME:
MAIN BUILDING MEZZANINE
PLAN / OFFICE BUILDING
ROOF PLAN

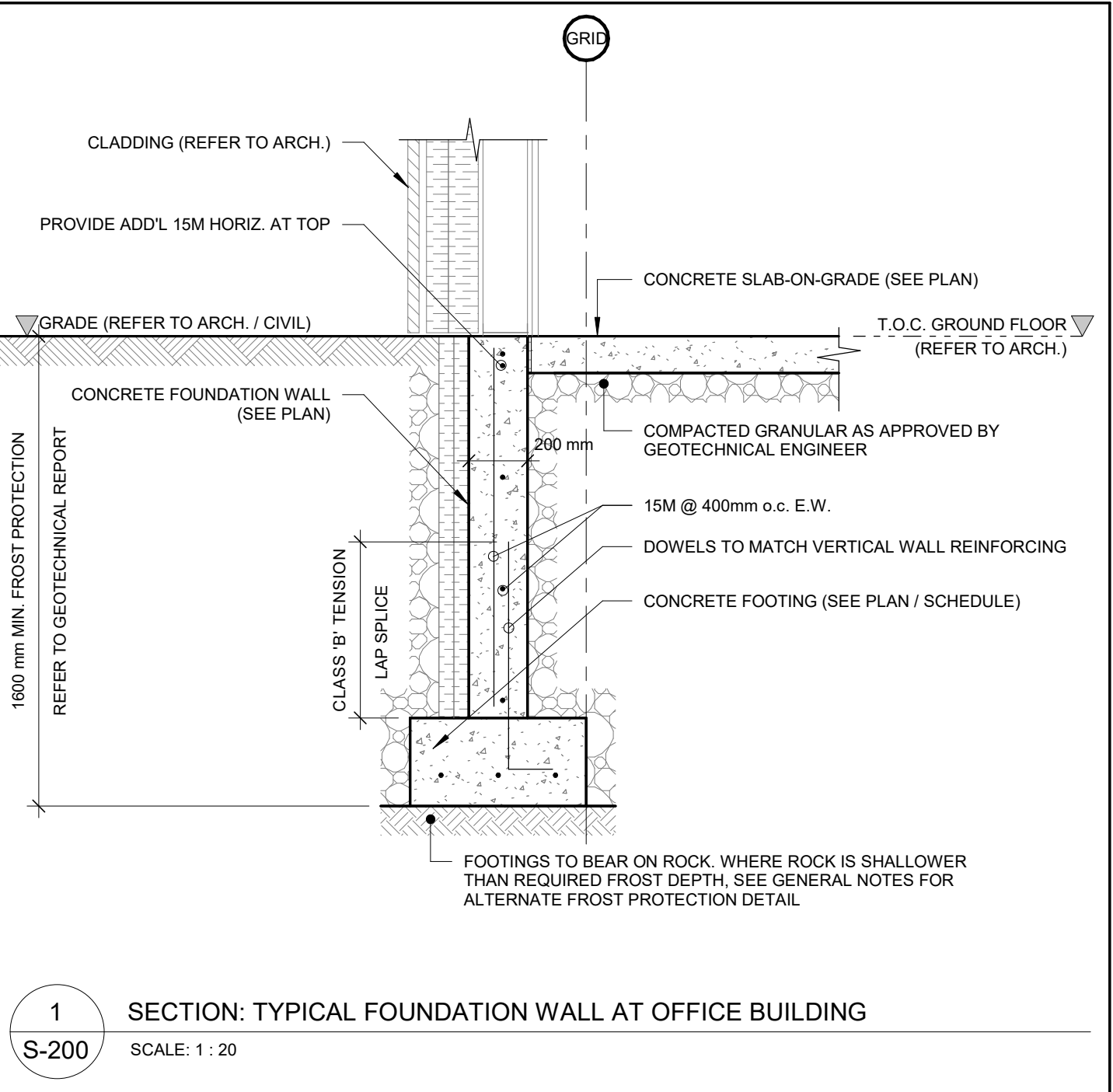
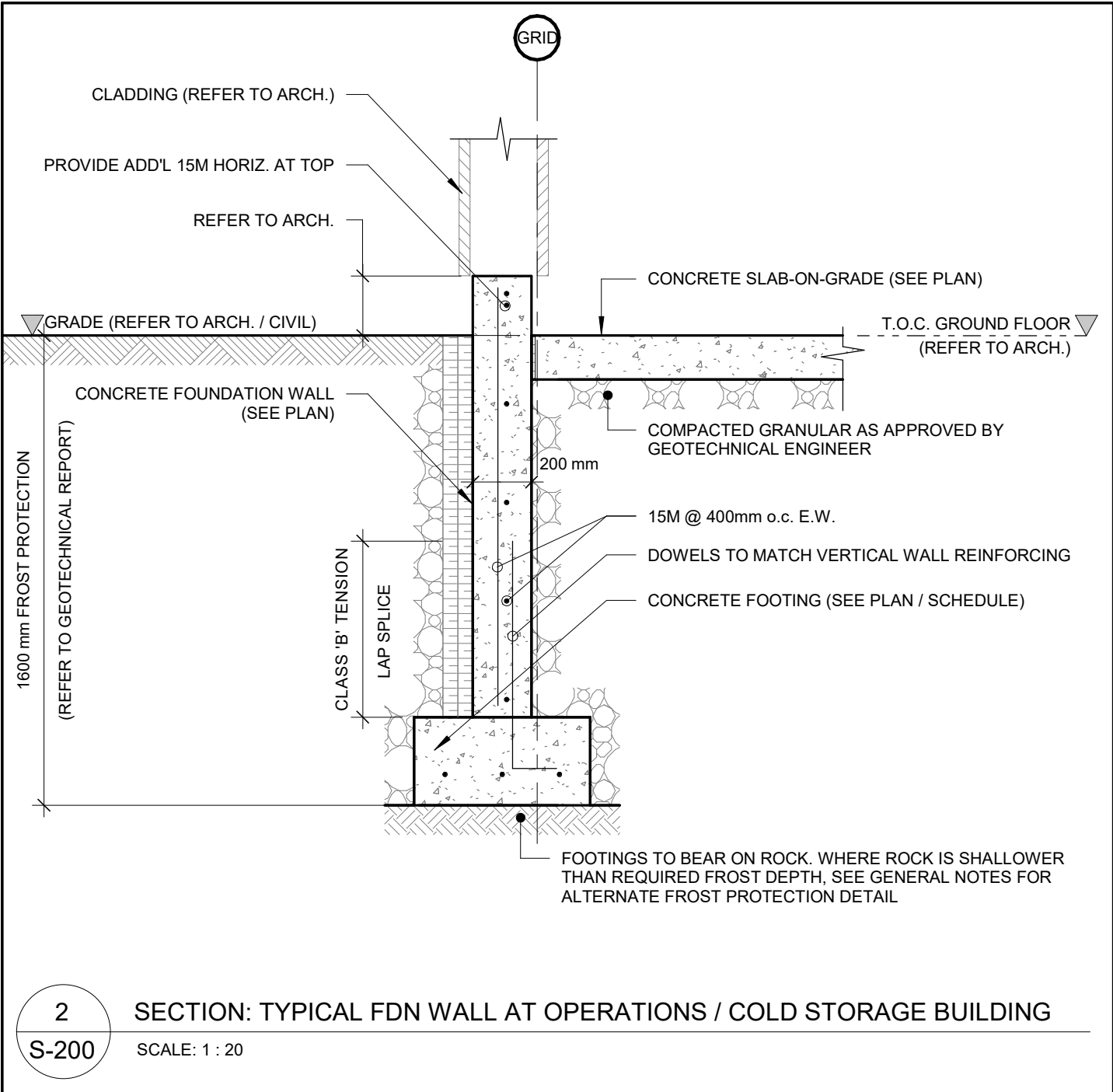
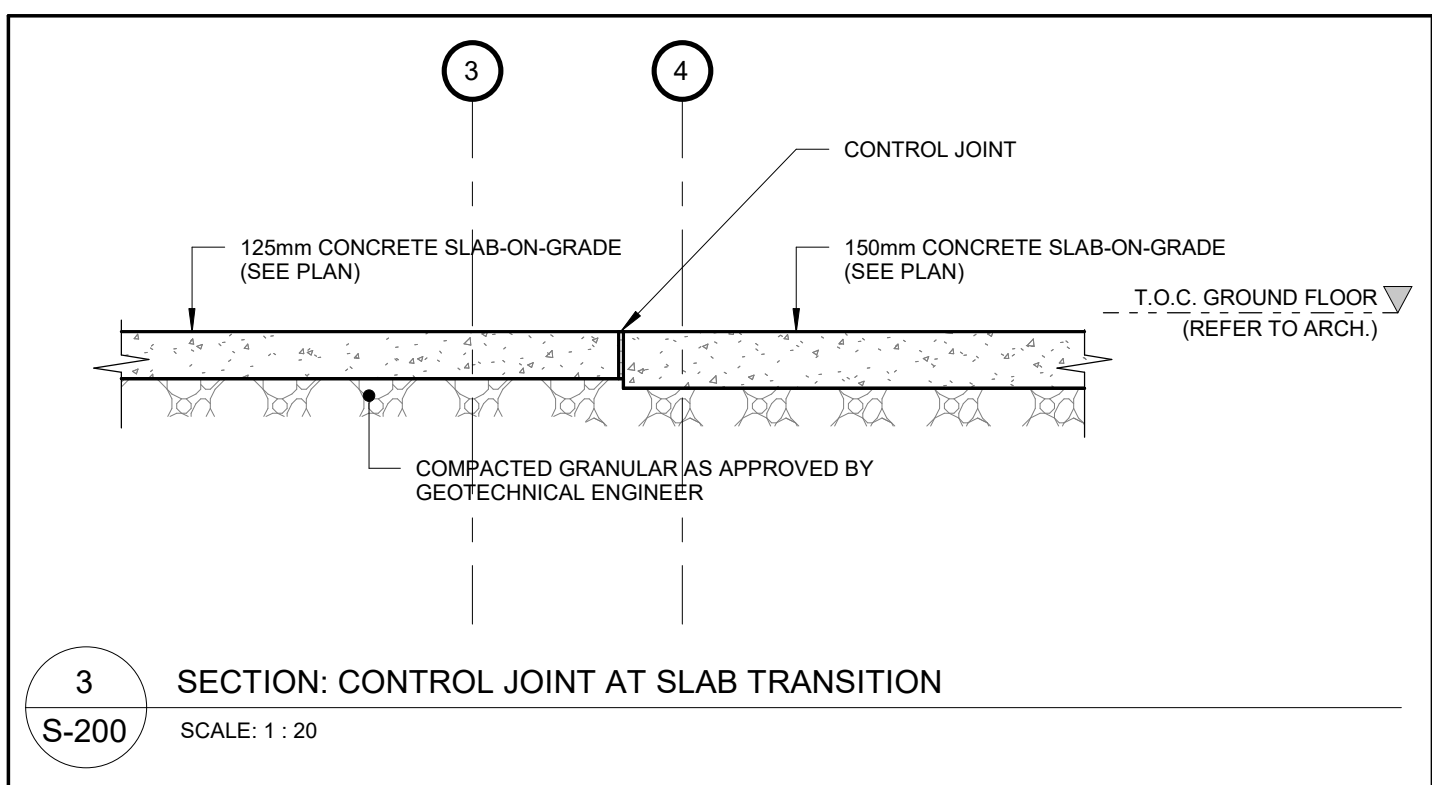
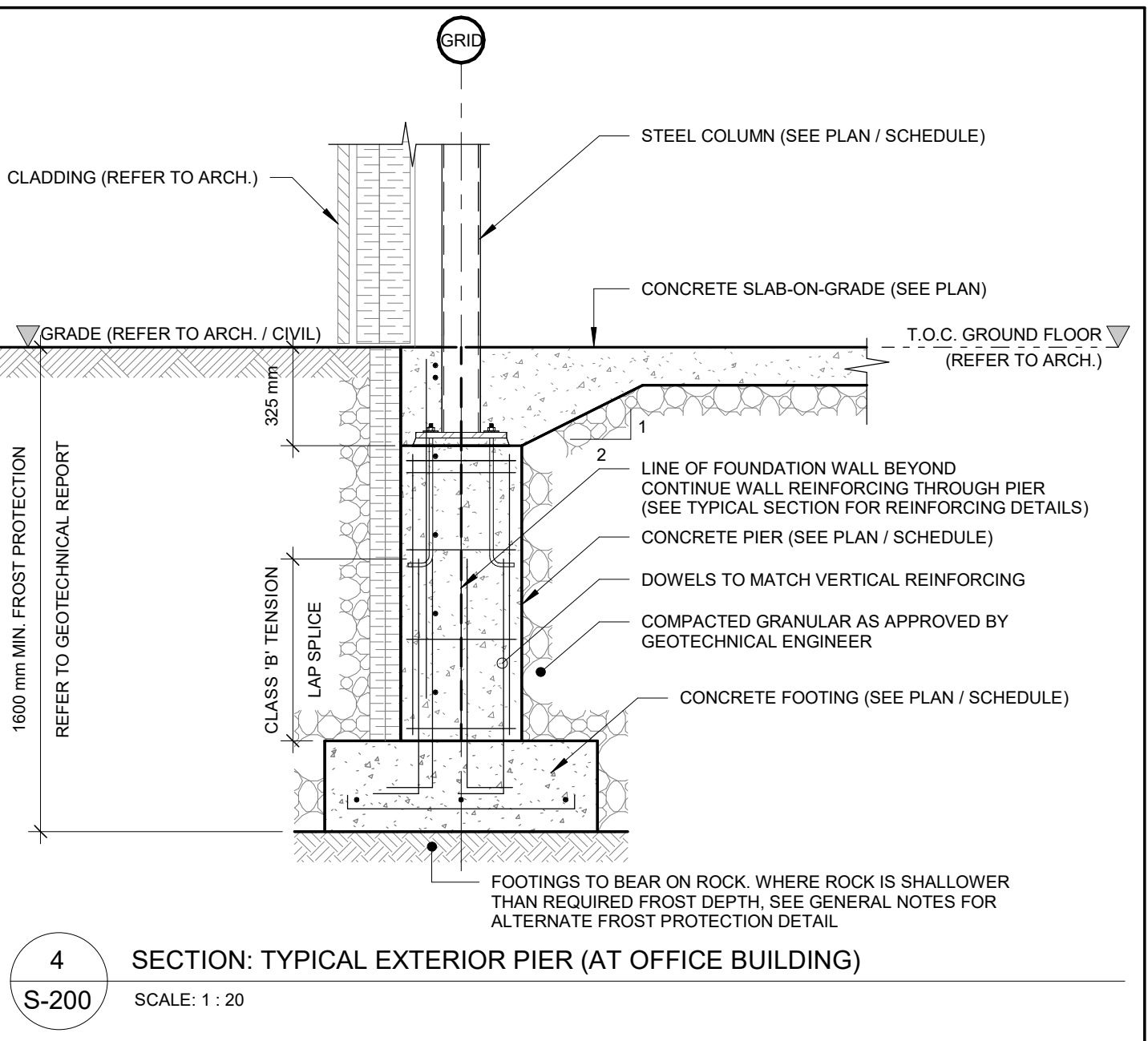
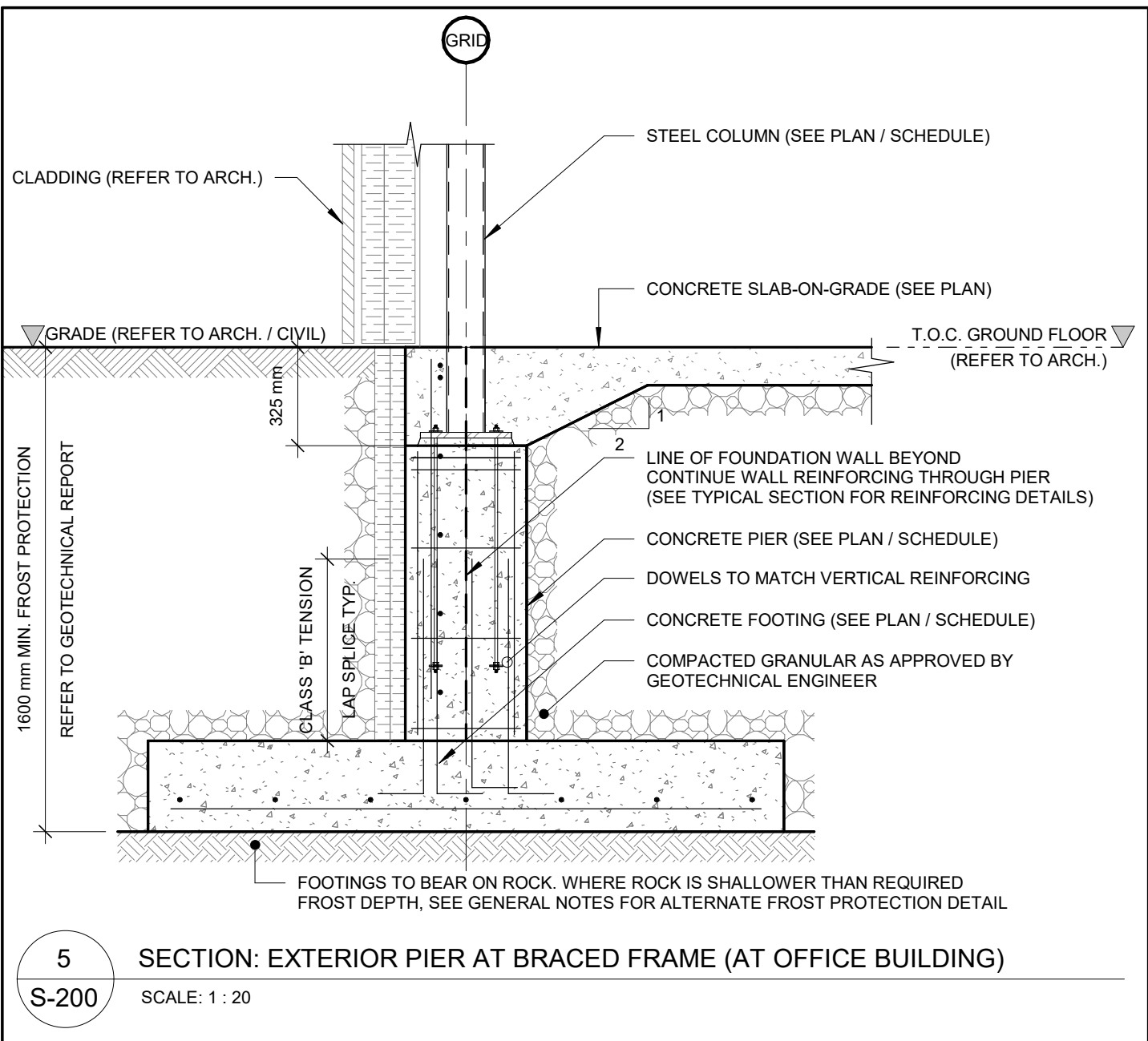
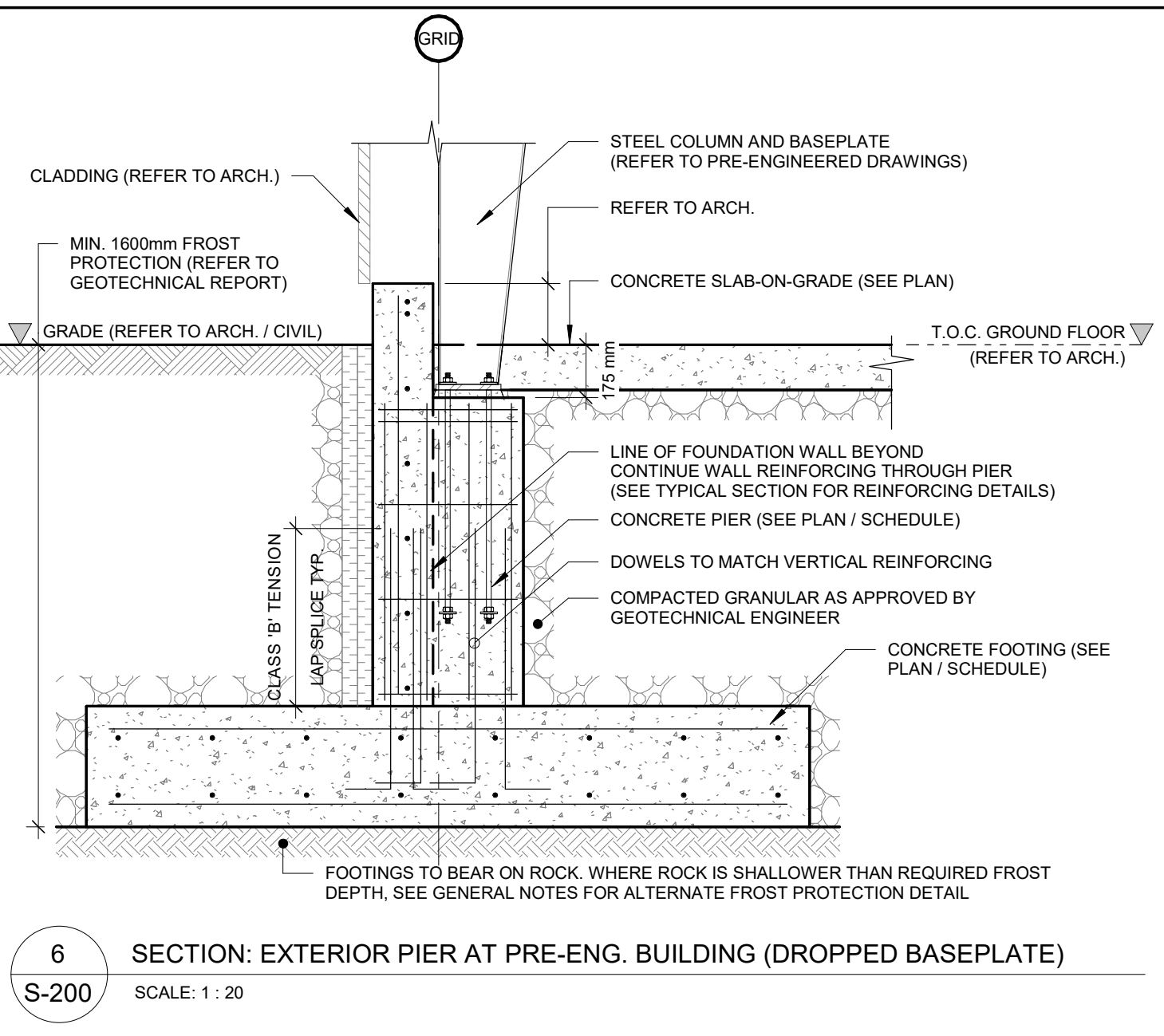
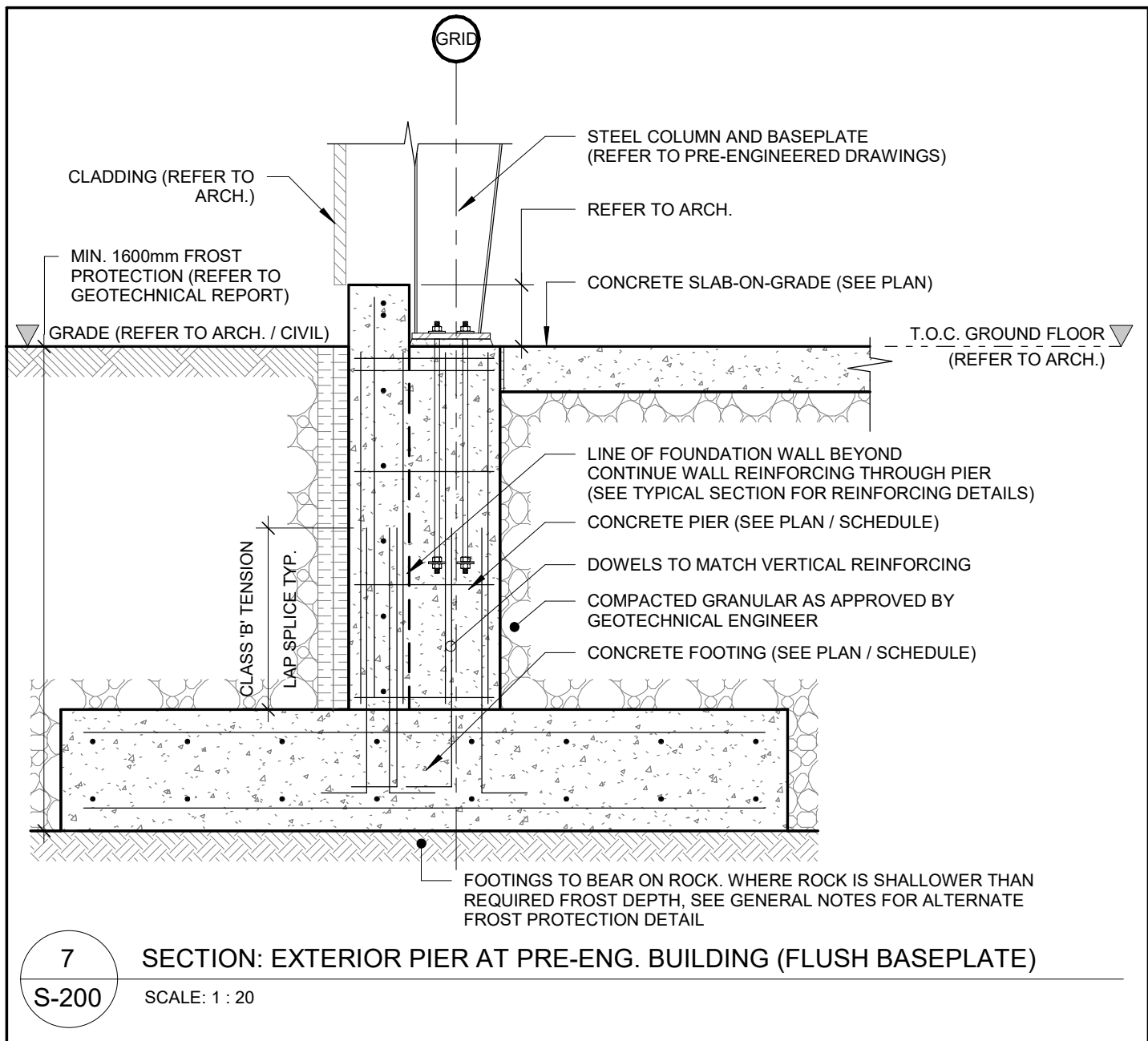
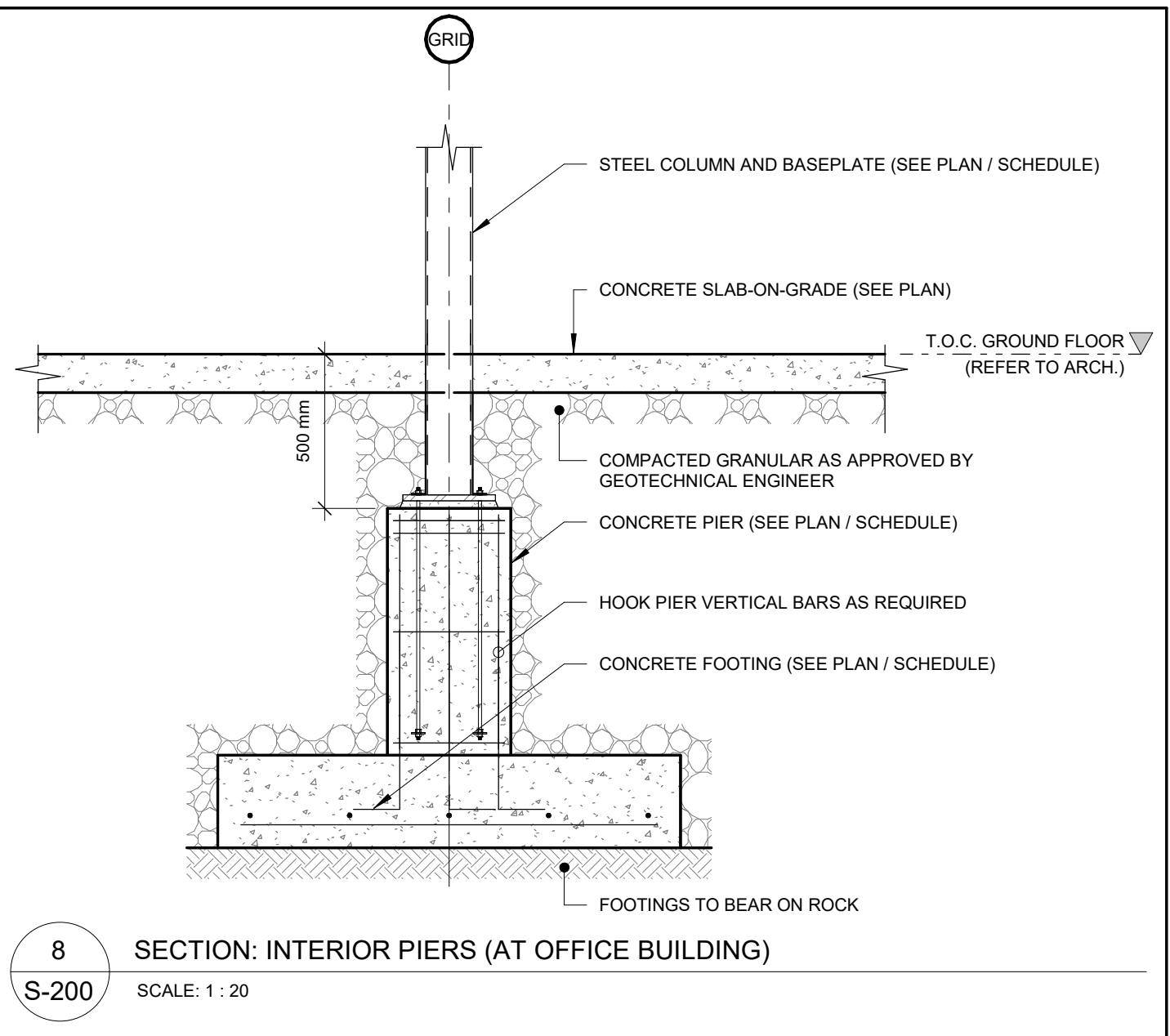
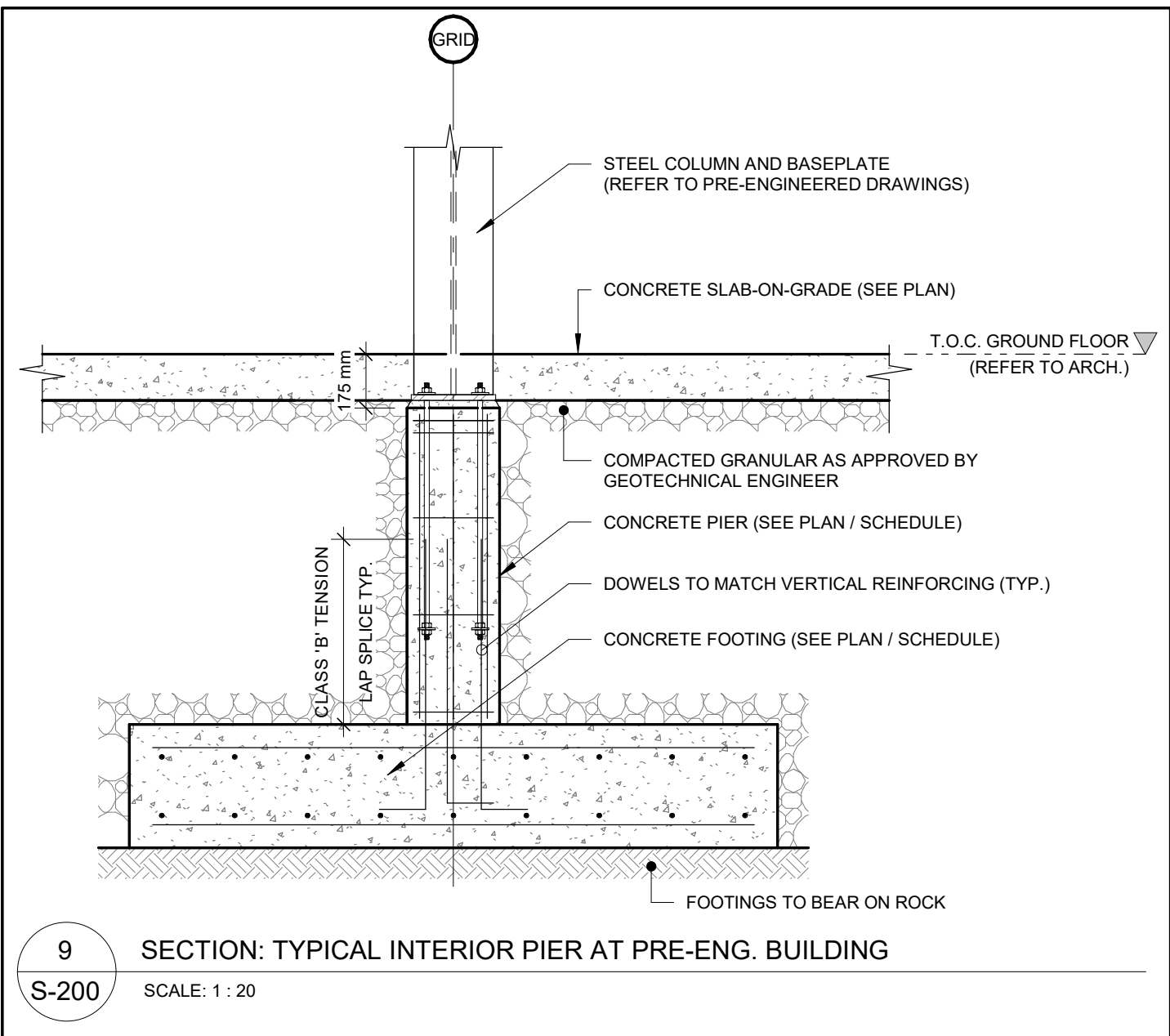
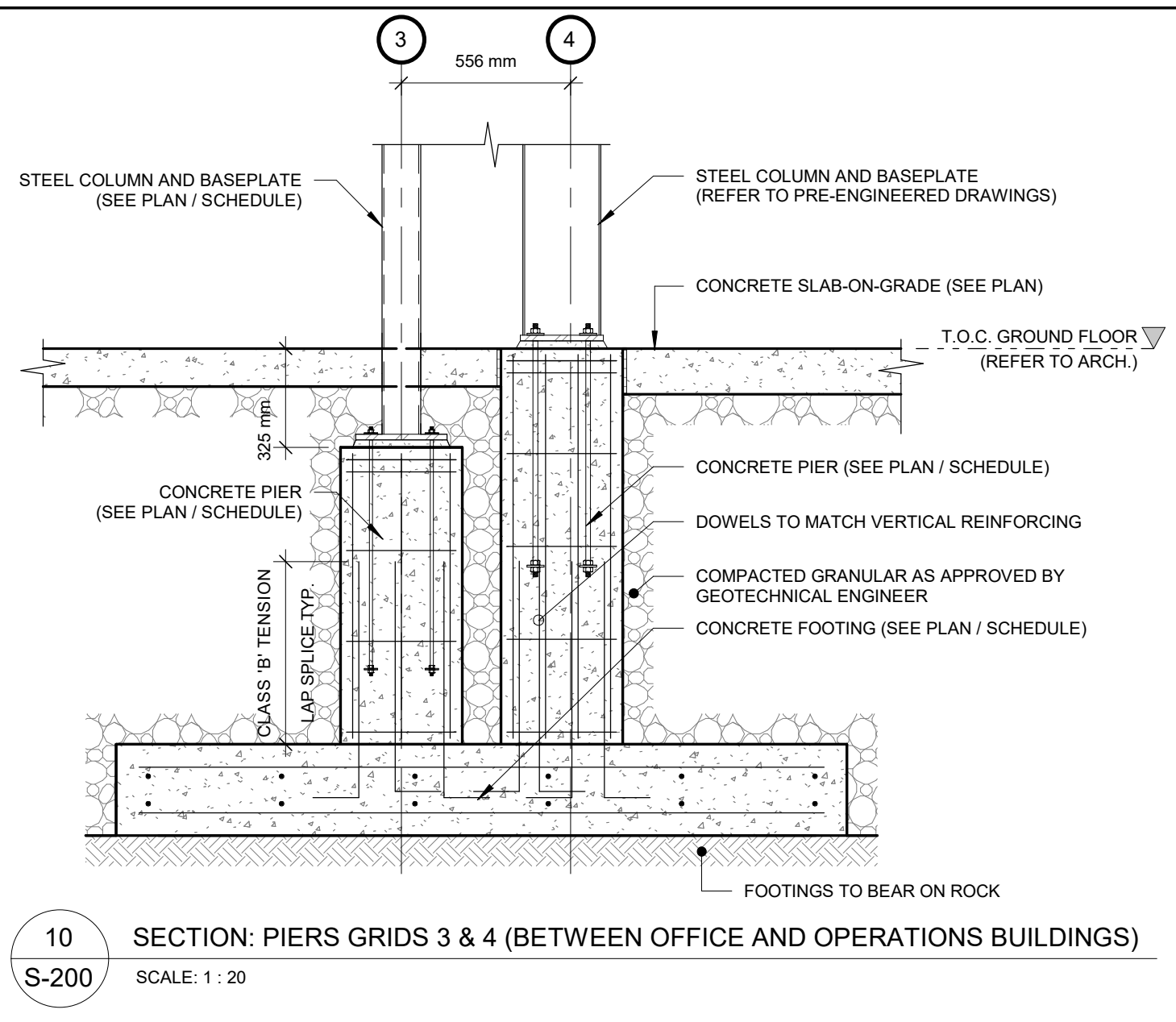
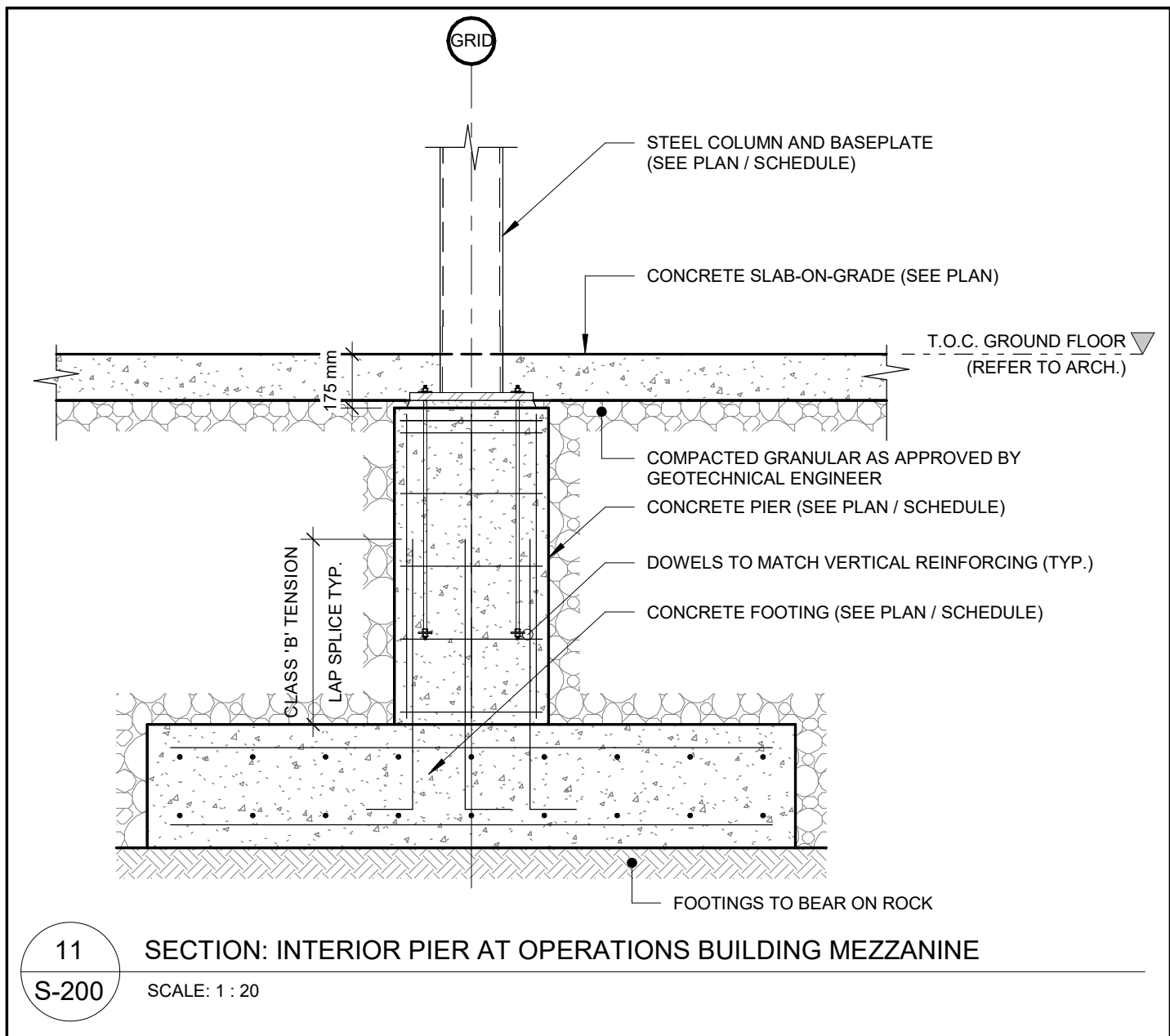
DESIGNED BY: S. HURD

DRAWN BY: J. LABBÉ

START DATE: 2026 05 29

PROJECT #: 26-09

S-102



3	2026 07 15	ISSUED FOR PERMIT / TENDER
2	2026 06 19	ISSUED FOR FINAL COORDINATION
1	2026 06 05	ISSUED FOR COORDINATION
#	DATE	DESCRIPTION

STAMP: NORTH ARROW:



PROJECT NAME AND ADDRESS:
TOWN OF SMITH FALLS
PUBLIC WORKS
OPERATIONS CENTRE

11 AIR SPACE DR.
SMITHS FALLS, ON. K7A 0B2

DRAWING NAME:
SECTIONS AND DETAILS

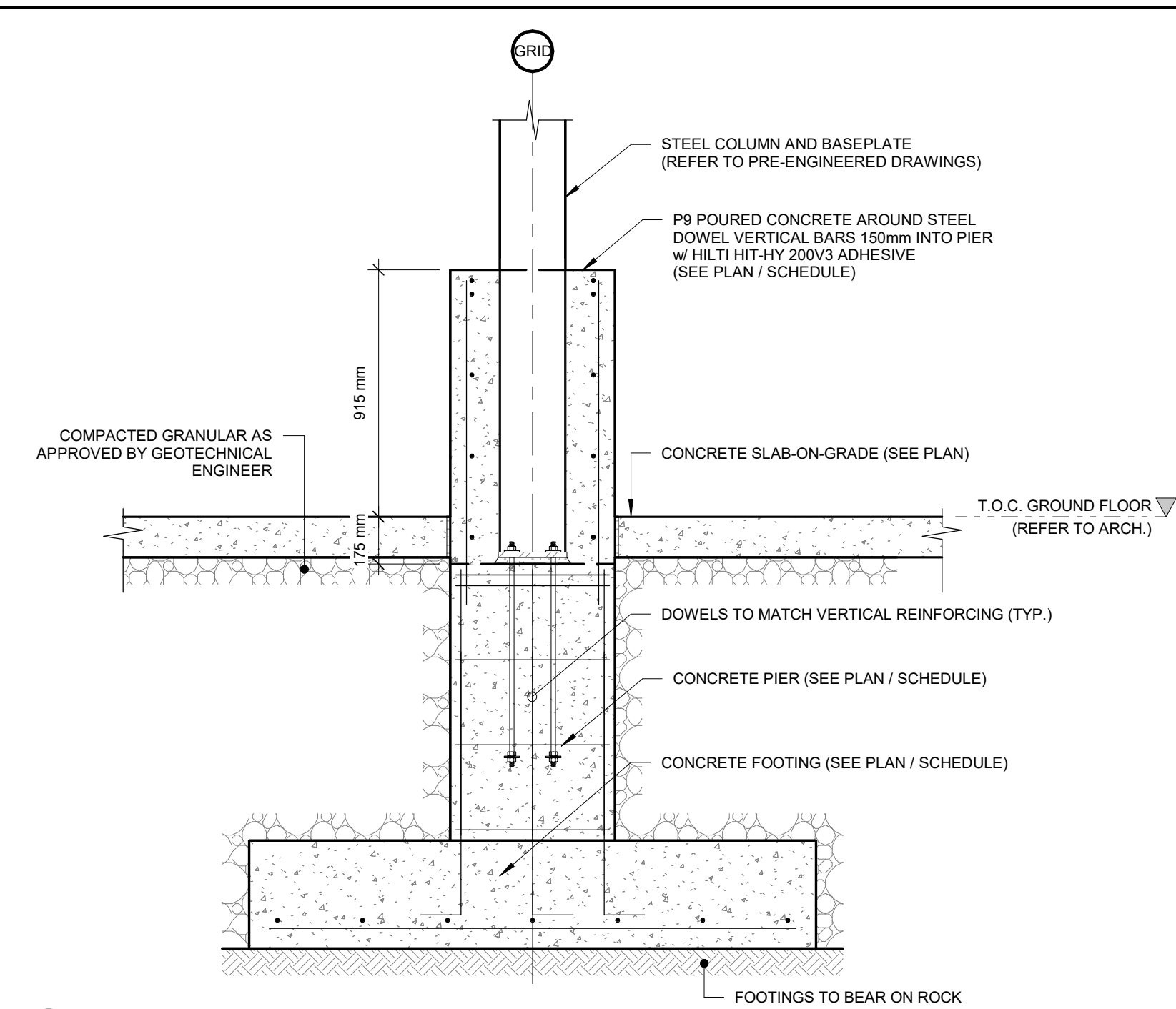
DESIGNED BY: S. HURD

DRAWN BY: J. LABBE

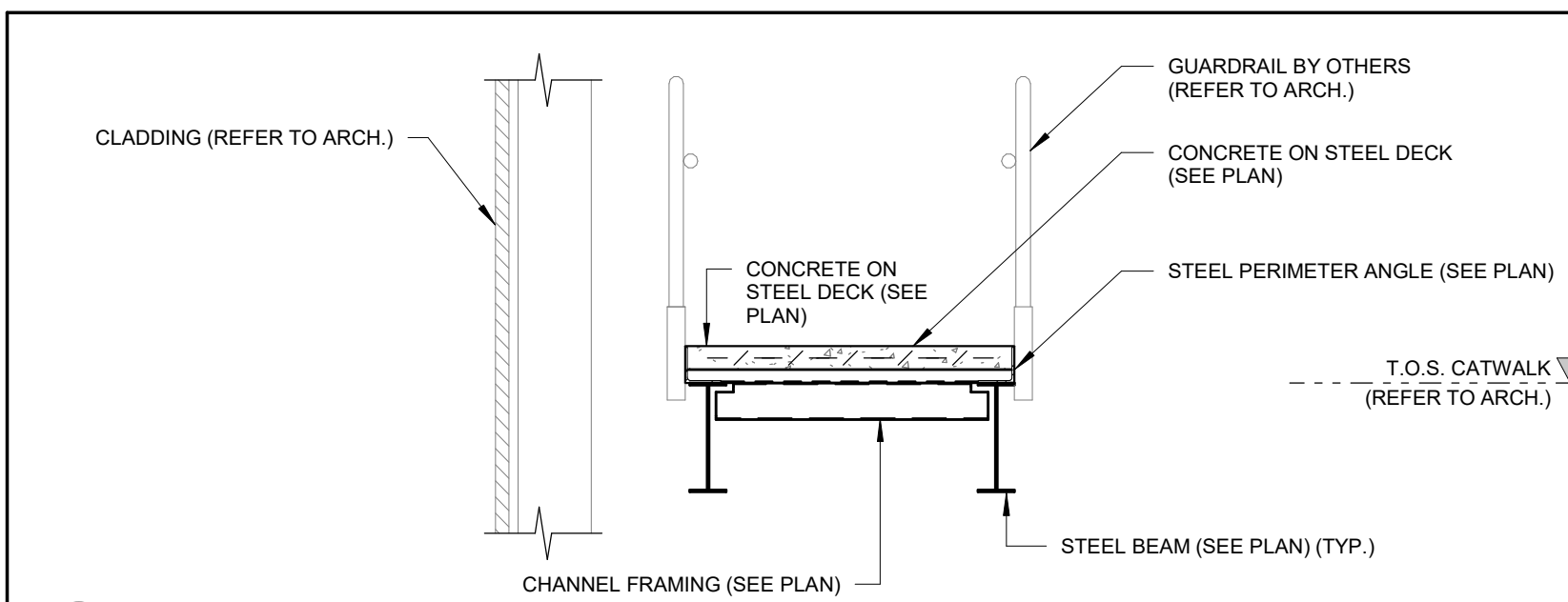
START DATE: 2026 05 29

PROJECT #: 26-097

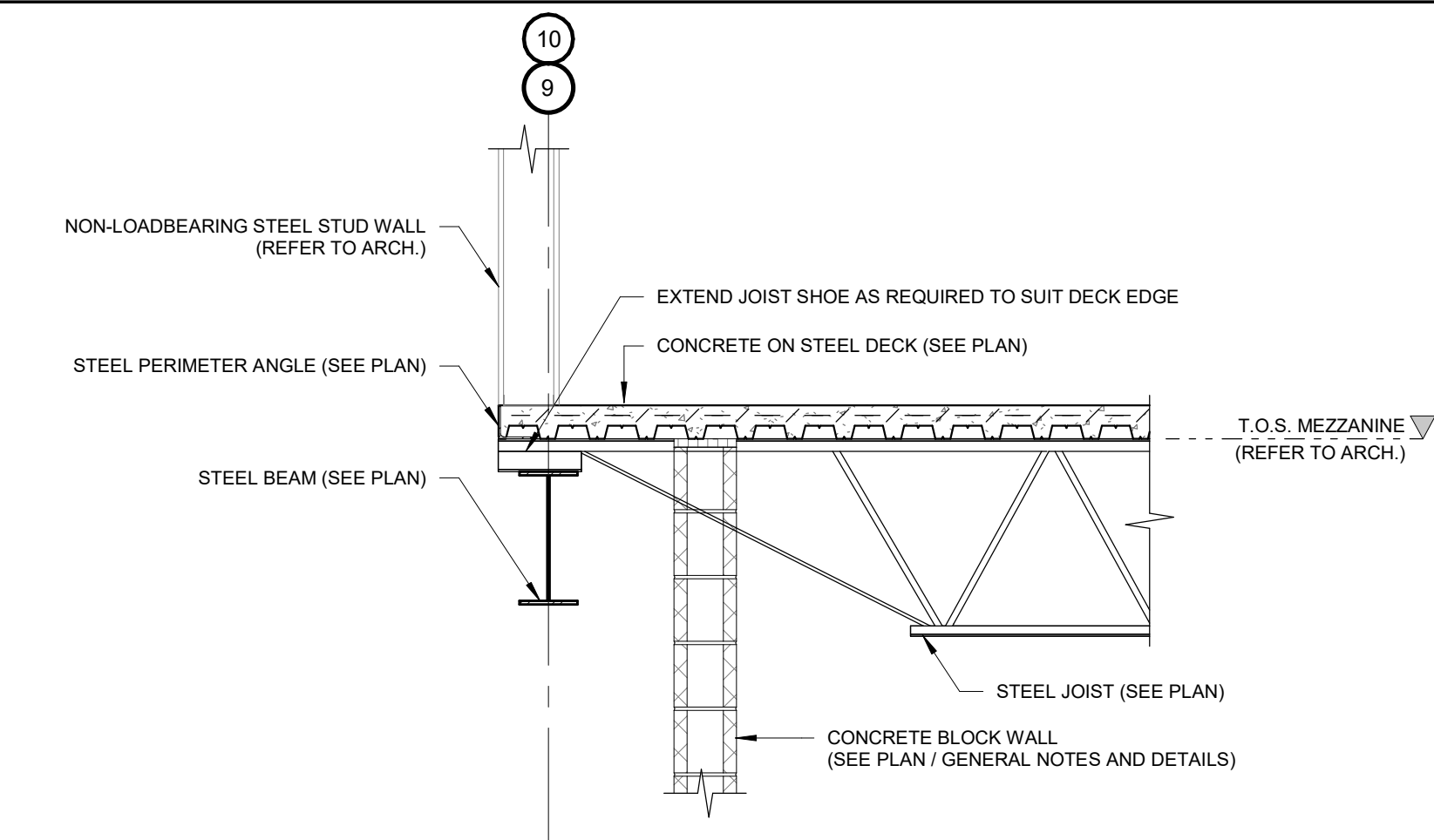
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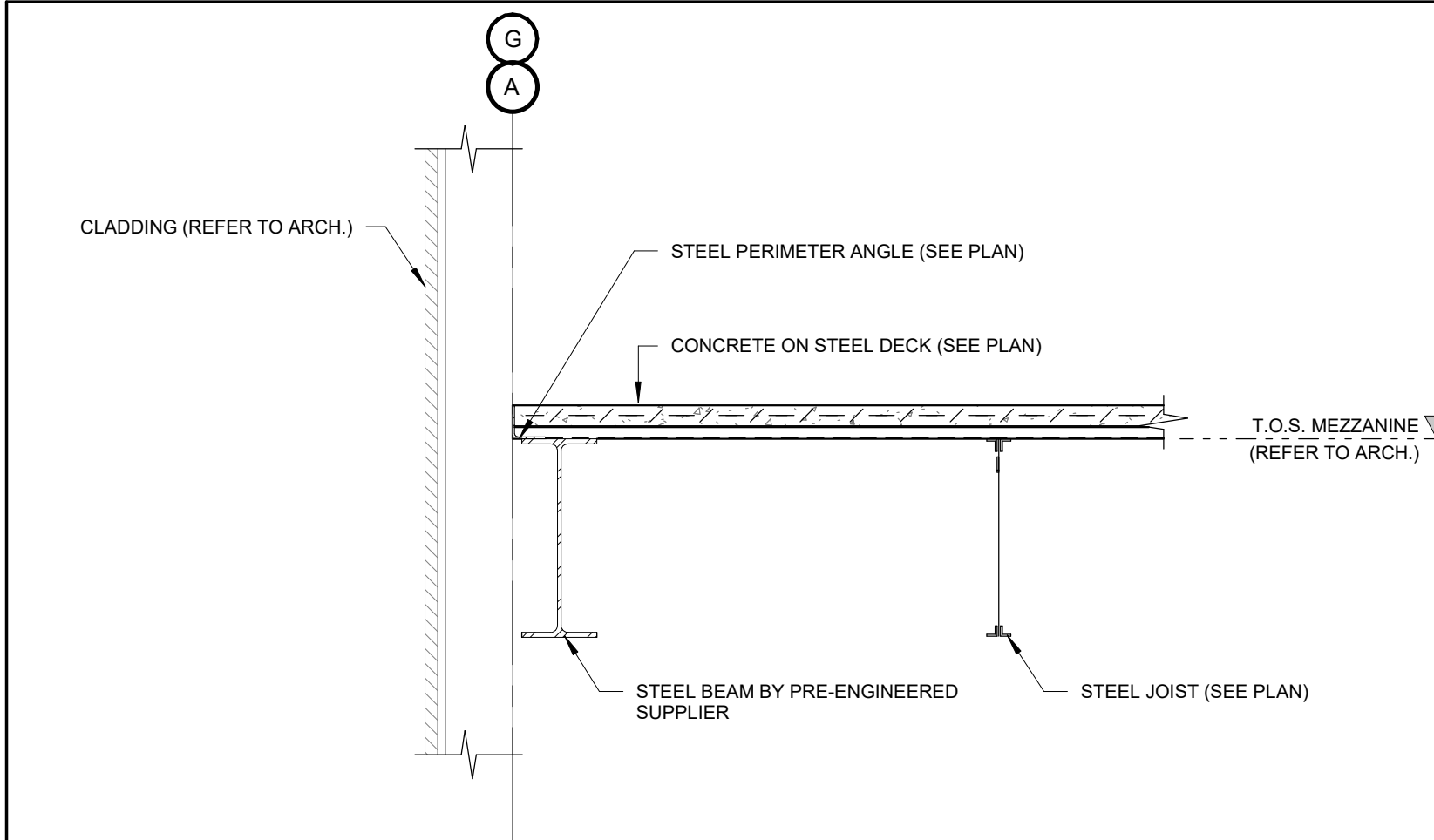
6 SECTION: COLUMN PROTECTION AT OPERATIONS BUILDING (GRID D5, D6, D7 AND D8)
S-201 SCALE: 1 : 20



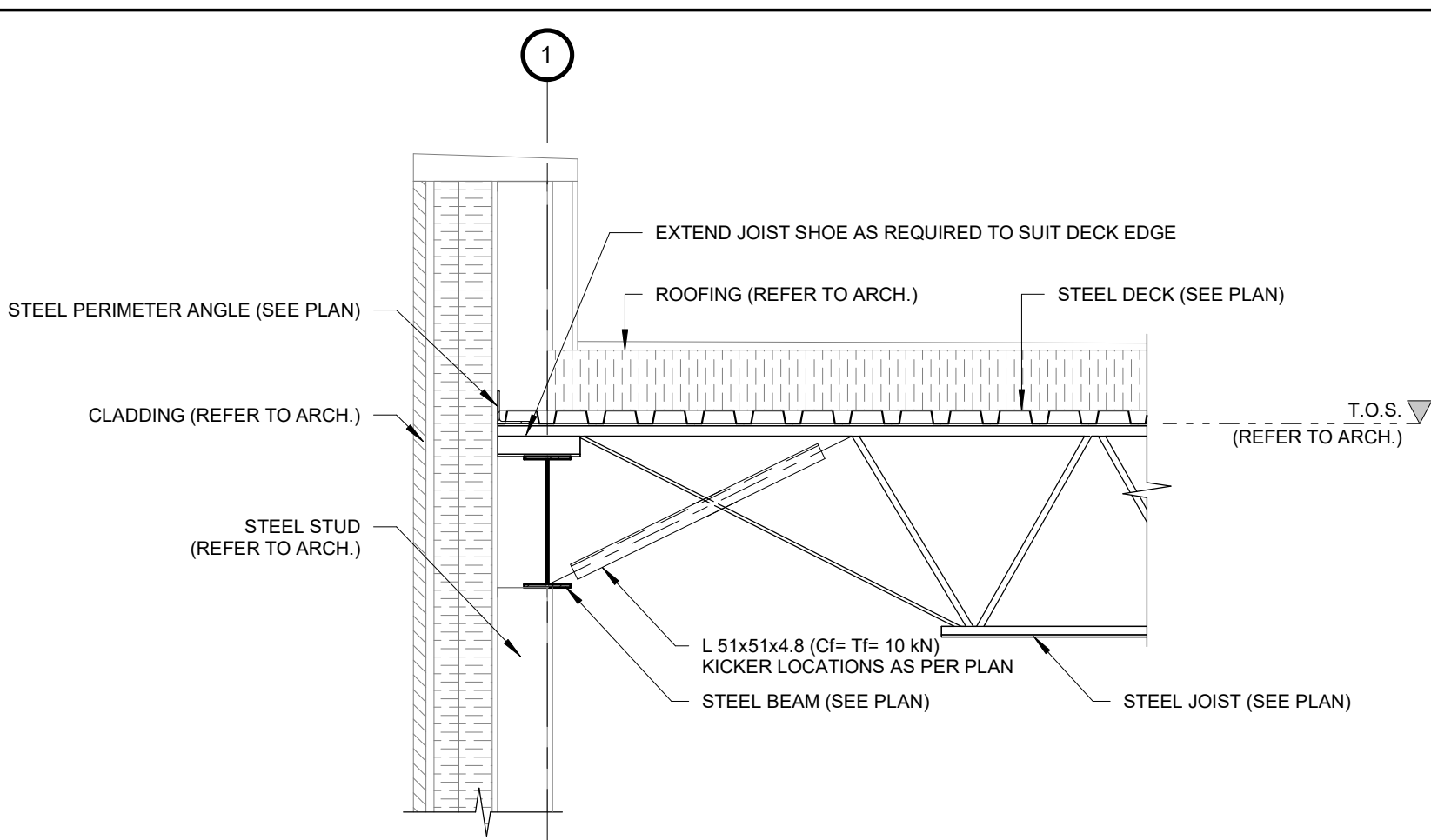
7 SECTION: FRAMING AT CATWALK
S-201 SCALE: 1 : 20



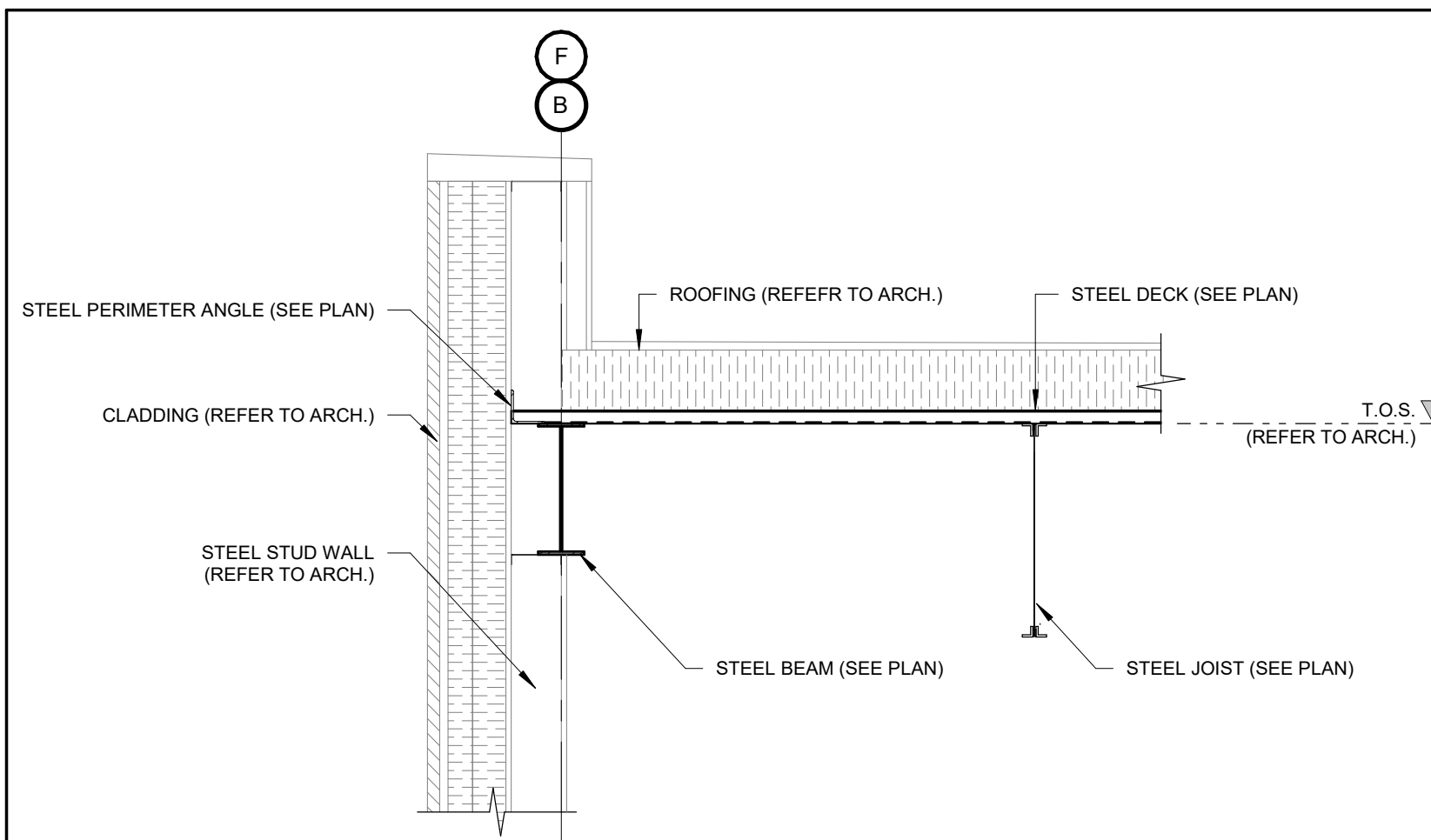
4 SECTION: DECK EDGE AT MEZZANINE (JOIST PERPENDICULAR TO PERIMETER BEAM)
S-201 SCALE: 1 : 20



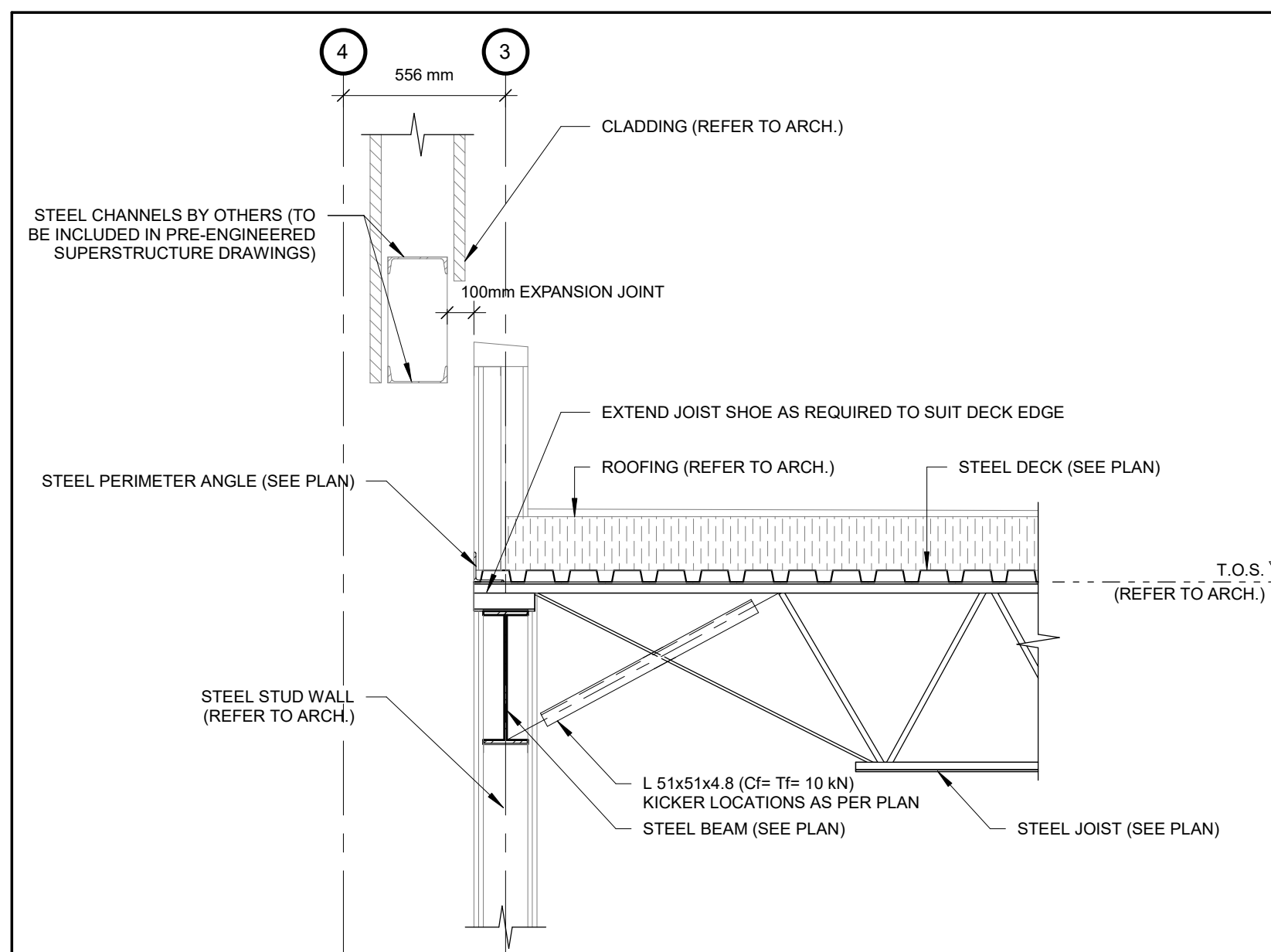
5 SECTION: DECK EDGE AT MEZZANINE (AT GRIDS A AND G)
S-201 SCALE: 1 : 20



1 SECTION: ROOF DECK EDGE AT GRID 1 (JOIST PERPENDICULAR TO PERIMETER BEAM)
S-201 SCALE: 1 : 20



2 SECTION: ROOF DECK EDGE AT GRIDS B AND F (JOIST PARALLEL TO PERIMETER BEAM)
S-201 SCALE: 1 : 20



3 SECTION: ROOF DECK EDGE AT GRID 3
S-201 SCALE: 1 : 20

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11 AIR SPACE DR.
SMITHS FALLS, ON. K7A 0B2

DRAWING NAME:

SECTIONS AND DETAILS

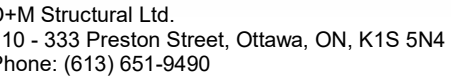
DESIGNED BY: S. HURD

DRAWN BY: J. LABBE

START DATE: 2026 05 29

PROJECT #: 26-097

S-201



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1. ALL FORCES NOTED ARE UNFACTORED AND HAVE NOT BEEN AMPLIFIED BY Rd.
2. MOMENTS GIVEN AT ENDS OF BEAMS ARE MAX. UNFACTORED / UNAMPLIFIED REACTIONS IN EACH DIRECTION.
3. SFRS: STEEL MOMENT FRAMES RESISTING FRAME: CONVENTIONAL CONSTRUCTION ($R_d = 1.5$, $R_o = 1.3$)
4. COLLECTOR FORCES ARE SHOWN ON PLAN.

1. **+/- 100 kN** DENOTES AXIAL LOAD (kN) IN BRACE / BEAM (+/-).
2. COORDINATE OPENING LOCATIONS AND TOP OF STEEL ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
3. SOME ROOF STRUCTURE NOT SHOWN FOR CLARITY.

1. MOMENTS AND FORCES GIVEN ON THIS DRAWING ARE SEISMIC (AXIAL) DESIGN LOADS. FOR CONNECTION DESIGN, THESE FORCES SHOULD BE AMPLIFIED BY R_d AS PER CSA S16 CL. 27.11.
2. SFRS: CONVENTIONAL CONSTRUCTION ($R_d = 1.5$).
3. COLLECTOR FORCES ARE SHOWN ON PLAN.



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TOWN OF SMITH FALLS
PUBLIC WORKS
OPERATIONS CENTRE

11 AIR SPACE DR.
SMITHS FALLS, ON. K7A 0B2

DRAWING NAME:
BRACED FRAME
ELEVATIONS

DESIGNED BY: S. HURD

DRAWN BY: J. LABBÉ

START DATE: 2026 05 29

PROJECT #: 26-097

S-300

PART 1 SUPPLEMENTAL GENERAL CONDITIONS

1.1 INTRODUCTION

- .1 The following Supplementary Conditions modify, change, delete from or add to the Articles of Agreement, the Definitions, and the General Provisions of the Stipulated Price Contract

1.2 MODIFICATIONS

AMENDMENTS TO THE CONTRACT

PART 13 INDEMNIFICATION AND WAIVER

GC 13.1 INDEMNIFICATION

ADD the following general condition

GC 13.1.7 *The Contractor shall defend, indemnify and save harmless the Corporation of the Town of Smiths Falls, its elected officials, officers, employees and agents from and against any and all claims of any nature, actions, causes of action, losses, expenses, fines, costs (including legal costs), interest or damages of every nature and kind whatsoever, including but not limited to bodily injury, sickness, disease or death or to damage to or destruction of tangible property including loss of revenue or incurred expense resulting from disruption of service, arising out of or allegedly attributable to the negligence, acts, errors, omissions, misfeasance, nonfeasance, fraud or willful misconduct of the Contractor, its directors, officers, employees, agents, contractors and subcontractors, or any of them, in connection with or in any way related to the delivery or performance of this Contract. This indemnity shall be in addition to and not in lieu of any insurance to be provided by the Contractor in accordance with this Contract and shall survive this Contract.*

GC 13.1.8 *The Contractor agrees to defend, indemnify and save harmless the Corporation of the Town of Smiths Falls from and against any and all claims of any nature, actions, causes of action, losses, expenses, fines, costs (including legal costs), interest or damages of every nature and kind whatsoever arising out of or related to the Contractor's status with WSIB. This indemnity shall be in addition to and not in lieu of any proof of WSIB status and compliance to be provided by the Contractor in accordance with this Contract and shall survive this Contract.*

END OF SECTION